MACKENZIE.

TRANSPORTATION IMPACT ANALYSIS

To

City of Manzanita

For

Oregon Coast Development

Dated

November 13, 2024

Project Number 2160454.11



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I. INTRODUCTION

This Transportation Impact Analysis (TIA) has been prepared in support of the proposed Manzanita Pines residential project in Manzanita, Oregon. Figure 1 in Appendix A presents a vicinity map indicating the project location.

Project Description

The proposed Manzanita Pines residential project located on the proposed Loop Road connected to Necarney City Road in Manzanita, Oregon will include multiple phases of development. Phase 1 will include 60 residential units between one- and three-bedroom and 500-1,200 square feet (SF) in size. The project will also include a common building, plaza, and playground. The apartments are intended to be an affordable option for local residents.

Scope of Analysis

This TIA has been prepared in accordance with the ODOT APM Version 2 and the scoping memo from Lancaster Mobley dated October 2, 2024. This TIA includes a summary of existing traffic conditions, proposed trip generation, trip distribution and assignment, crash review, an analysis of intersection operations, and queuing. The scoping letter is provided in Appendix B.

Study Area

This TIA includes a study of the following City of Manzanita intersections:

- Necarney City Road/Highway 101
- Necarney City Road/Loop Road

Analysis Scenarios

Analysis is provided for all study area intersections. Construction is anticipated at the end of 2025, so this study assumes cull occupancy in 2026. This TIA addresses transportation conditions for the following analysis scenarios during the PM peak hours and Saturday peak hours:

- 2024 Existing
- 2026 Pre-Development without Manzanita Pines
- 2026 Post-Development with Manzanita Pines



II. EXISTING CONDITIONS

The existing conditions analysis is based on a current year inventory of transportation facilities and traffic data collected on October 10 and 12, 2024.

Site Conditions

The project site is located on the north side of Necarney City Road between Clipper Court and Pine Ridge Drive in Manzanita, Oregon. The site is zoned R2, Medium Density Residential. The site is currently vacant.

Vehicular Transportation Facilities

The study area presented in this TIA includes roadways under City of Manzanita as well as ODOT jurisdiction. Figure 3 presents the existing lane configurations and traffic control devices for the study area intersections (Appendix A).

Table 1 summarizes the characteristics of the study area roadways.

TABLE 1 – ROADWAY CHARACTERISTICS								
Roadway	Functional Classification	Posted Speed (mph)	Travel Lanes	Lane Width	Bike Lanes	On-Street Parking	Sidewalks	
Necarney City Road	Minor Collector	35	2	12 feet	No	No	No	
Highway 101	Principal Arterial/ Statewide Highway	40	2	12 feet	No	No	No	
Loop Road	Local Street	25	2	10 feet	No	No	No	

Pedestrian and Bike Facilities

Bike lanes and sidewalks are not currently provided on any of the area roadways as noted above.

Transit Facilities

The City of Manzanita is part of the NWConnector transit system. Route 3 provides service to Manzanita as it passes between Cannon Beach and Tillamook. The greater NWConnector transit system provides connections between Astoria to the north and Yachats to the south along Highway 101. It also provides connections to the east, from Kelso, Washington to the north to Albany, Oregon to the south, primarily along the I-5 corridor. A copy of the NWConnector Route 3 schedule and map have been provided in Appendix C.

Existing Traffic Counts

Turning movement counts utilized in this study were collected on Thursday, October 10, 2024, and Saturday, October 12, 2024. Because the Loop Road intersection with Necarney City Road does not yet exist and to obtain data on residential trip distribution, we collected data at the intersection of Pine Ridge



and Necarney City Road. Figure 4 presents the existing PM peak hour and Saturday peak hour traffic volumes for all study area intersections (Appendix A). Raw traffic count summaries are provided in Appendix D.

Seasonal Adjustment

Seasonal adjustment factors were reviewed using ODOT's ATR Seasonal Trend Table for coastal destination with a count conducted on October 10 and 12. The calculated seasonal adjustment factor of 1.19 was applied to the 2024 existing traffic counts at all locations. Figure 5 presents the seasonally adjusted 2024 volumes (Appendix A).

Crash Analysis

Historical crash data reported for the study area were evaluated for safety. Crash data for the 5-year period of 2018 through 2022 were obtained from ODOT and used to review crash patterns and estimate crash rates for the study area intersection of Necarney City Road with Highway 101 and along Necarney City Road between Highway 101 and Classic Street. Two crashes were noted at the intersection and four crashes along the approximately one-mile segment of Necarney City Road.

Intersection Crash Rates

When evaluating the relative safety of an intersection, consideration is given not only to the total number and types of crashes occurring, but also to the number of vehicles entering the intersection. This concept, referred to as a "crash rate," is usually expressed in terms of the number of crashes occurring per one million entering vehicles (MEV) for the intersection per year. Intersections having a crash rate higher than 1.0 crashes/MEV should be reviewed for opportunities to improve safety.

The intersection crash rate is calculated by dividing the average number of crashes per year by the MEV per year. A daily traffic volume was estimated by dividing the PM peak hour volume at the intersection by a peak-to-daily factor, or k-factor. A k-factor of 0.144 from ODOT traffic data taken 0.1 miles east of Necarney City Road on Highway 101 was found on ODOT's TransGIS web portal was applied to the PM peak hour traffic volume collected on October 10, 2024, to estimate ADT.

Road segment crash rates are calculated similarly to intersections but are based on the vehicle miles traveled. The number of crashes is divided by the vehicle volume times the length of the segment and is expressed in crashes per million vehicle miles traveled (MVMT). The daily volume on Necarney City Road was estimated by applying the same k-factor to the PM peak hour volume just south of the intersection with Highway 101.

The raw crash data and calculations is provided in Appendix F.

Crash Data Summary

There were two crashes reported at the intersection of Necarney City Road with Highway 101. One was a fixed-object crash in 2018 caused by an improper westbound left turning movement, resulting in a suspected minor injury (Injury Type B). The other was a crash with a cyclist caused by a failure to yield by the driver at fault, resulting in a suspected serious injury (Injury Type A). With an estimated daily volume of 5,000 vehicles, the resulting crash rate is 0.18 crashes per MEV. This is much less than ODOT's 90th Percentile rate of 0.475 for similar intersection types.



There were four crashes reported along Necarney City Road between Highway 101 and Classic Street. All four appear to be single-vehicle crashes caused by driver error (i.e., driving too fast for conditions, hitting a fixed object or deer/elk, etc.). None of the crashes occurred near the proposed Loop Road intersection. With an estimated daily volume 938 vehicles, the crash rate was calculated to be 2.34 crashes per MVMT.

Though the crash rate is high on the segment of Necarney City Road, it is generally the result of driver behavior such as driving too fast for conditions. None of the crashes were intersection-related and all involved a single vehicle. Therefore, we do not believe the added Loop Road intersection will have an impact on safety on the roadway. No further crash analysis is recommended.



III. PRE-DEVELOPMENT CONDITIONS

The pre-development condition reflects a buildout year scenario without the proposed development. This scenario includes traffic from the 2024 existing condition, background traffic growth to the year 2026, and in-process traffic from other approved developments that have not been constructed.

Planned Transportation Improvements

None noted in the study area.

Background Traffic Growth

Based on data from ODOT's 2040 Future Volumes Table from 0.2 miles north of Manzanita and 0.2 miles south of Laneda, and recent studies prepared in Manzanita, a 1% growth rate per year was applied to the study area intersections.

Figure 6 presents the PM peak hour and Saturday peak hour background traffic growth volumes for all study area intersections (Appendix A).

In-Process Traffic

In-process traffic volumes account for developments that have been approved or that are under construction at the time of the traffic counts. These traffic volumes account for trips that will be added to the external roadway network before build-out of the proposed development. Traffic volumes for the following developments were included in the analysis to account for in-process traffic:

- Manzanita Lofts
- Heron's Rest
- Nehalem Bay State Park Expansion

The detailed trip generation analysis for the Nehalem Bay State Park Expansion provided by ODOT via Lancaster Mobley listed zero trips on Saturday due to limited ITE data. The PM peak hour rate has been assumed for Saturday as a more appropriate estimate. Figure 7 presents the PM peak hour and Saturday peak hour in-process trips for the above project (Appendix A). Detailed information for the in-process projects is included in Appendix E.

Pre-Development Traffic

The 2026 pre-development analysis scenario is a combination of 2024 traffic volumes, a 1% annual background growth rate over two years, and in-process traffic. The pre-development traffic without the project trips will indicate if traffic issues are present before the addition of the proposed residential project.

Figure 8 presents the PM peak hour and Saturday peak hour 2026 pre-development traffic volumes (Appendix A).



IV. SITE DEVELOPMENT

The trip-making characteristics of the proposed development are described below.

Trip Generation

Trip generation estimates for the proposed project were developed using the Institute of Transportation Engineers' (ITE) *Trip Generation Manual,* 11th Edition. The ITE land uses that best match the proposed project is "Affordable Housing" (LUC 223). The data set for Saturday trip generation for "Affordable Housing" is limited, so the trip rates from "Multifamily Housing (Low-Rise)" (LUC 220) were used.

A trip generation summary is presented in Table 2.

TABLE 2 – TRIP GENERATION										
ITE Code	ITE Land Use	Size	Trip Type	PM In	Peak H Out	our Total	Saturd In	ay Peal Out	k Hour Total	Daily
223	Affordable Housing (Income Limit)	60	DU	15	13	28	15 ¹	10 ¹	25 ¹	289

¹Trip rates from "Multifamily Housing (Low-Rise)" (LUC 220) used

As shown in Table 2, the affordable housing development is expected to generate 28 PM peak hour, 25 Saturday peak hour, and 289 weekday daily trips.

Trip Distribution and Assignment

Trip distribution for the proposed development was estimated using similar studies for residential development and review of existing traffic volumes at the study area intersections and to the intersection of Necarney City Road with Pine Ridge Lane. Because of the nature of residential developments in this area, trip patterns differ between weekday and the weekend. The following trip distribution was used for PM peak hour trips:

- 5% to/from Nehalem Bay State Park
- 35% to/from Central Manzanita
- 20% to/from the north on Highway 101 via Necarney City Road
- 40% to/from the south on Highway 101 via Necarney City Road

The following trip distribution was used for Saturday peak hour trips:

- 5% to/from Nehalem Bay State Park
- 45% to/from Central Manzanita
- 15% to/from the north on Highway 101 via Necarney City Road
- 35% to/from the south on Highway 101 via Necarney City Road

Figure 9 presents the PM peak hour and Saturday peak hour site trip distribution and volumes (Appendix A).



Post-Development Traffic

Post-development traffic volumes are the sum of the site trips and the pre-development traffic volumes. Figure 10 presents the PM peak hour and Saturday peak hour 2024 post-development traffic volumes (Appendix A).



V. SITE ACCESS, CIRCULATION, AND PARKING

The evaluation of site access and on-site circulation are presented below. This evaluation includes assessment of sight distance.

Site Access and Circulation

The residential project will have access via two driveways on the proposed Loop Road. The southern driveway is proposed approximately 575 feet from Necarney City Road, and the second driveway is proposed 265 feet north of the first.

Sight Distance Evaluation

Based on the proposed Loop Road and project site, the site driveways on Loop Road will meet minimum stopping sight distance (SSD) and intersection sight distance (ISD) requirements per AASHTO design guidelines.

TABLE 3 – SIGHT DISTANCE EVALUATION									
Design Speed Recommended Required Available Sight Distance (feet									
Access	(MPH)	Design Vehicle	ISD (feet)	SSD (feet)	To North	To South			
South Access	25 MPH	Passenger Car	280	155	280	280			
North Access	25 MPH	Passenger Car	260	133	N/A	280			

The proposed intersection between Loop Road and Necarney City Road will address required sight distances through the design process.



VI. OPERATIONS ANALYSIS

Two aspects of operation analysis were evaluated for the study area intersections: 1) intersection operation analysis, which evaluates how well an intersection processes traffic demand; and 2) queuing analysis, which compares intersection queues with available storage for different travel lanes.

Intersection Operations Analysis

Intersection operations are generally measured by three mobility standards: volume-to-capacity (v/c) ratio, level-of-service (LOS), and delay (measured in seconds).

- V/C ratio is a measurement of capacity used by a given traffic movement or for an entire intersection. It is defined by the rate of traffic flow or traffic demand divided by the theoretical capacity calculated for the roadway geometry and traffic control.
- LOS is an expression of the average control delay (in seconds) experienced by drivers as described by a letter on the scale from A to F. LOS A represents optimum operating conditions and minimum delay, while LOS F indicates lengthy delays and often over-capacity conditions.
- Delay is a measurement of the average vehicle delay resulting from the type of traffic control and the conflicting traffic volumes. An average delay can be expressed for a certain movement, a specific lane, a single approach, or for an entire intersection.

Performance Measures

The Oregon Highway Plan (OHP) designates Highway 101 as a statewide highway that is Non-MPO outside of a Special Transportation Area. With a posted speed of 40 mph Table 6 of the OHP states the mobility target for the Highway 101 and Necarney City Road intersection is a v/c ratio of 0.85 or less.

The City of Manzanita has no clear operational standards for City intersections. It is assumed a level of service "D" or better would be sufficient for City intersections.

Methodology

Intersection operations were analyzed with the use of Synchro 11 software, which utilizes the Transportation Research Board's *Highway Capacity Manual* (HCM) 2000, HCM 2010, and HCM 7 methodologies. All the study area intersections are stop controlled.

Findings

The operation results for the worst-operating movement at each intersection are presented in Table . HCM 2000 and seven reports have been made available in Appendix G.



TABLE 4 – PEAK HOUR INTERSECTION OPERATIONS									
		Analysis Re	Analysis Results (v/c-LOS-Delay in seconds)						
Intersection (Control)	Peak Hour	2024 Existing	2026 Pre- Development	2026 Post- Development					
Necarney City Road/Hwy 101	PM	0.19-B-13.3 (NB)	0.23-B-14.2 (NB)	0.24-B-14.2 (NB)					
(Stop)	SAT	0.30-C-16.1 (NB)	0.34-C-17.2 (NB)	0.36-C-17.6 (NB)					
Necarney City Road/Loop Road	PM	N/A	N/A	0.02-A-9.3 (SB)					
(Stop)	SAT	N/A	N/A	0.01-A-9.7 (SB)					

As presented in Table 4, all study area intersections currently operate within ODOT and City standards and are projected to continue meeting standards under post-development conditions.

Intersection Queuing Analysis

An intersection queuing analysis was conducted for the study area intersections during the PM peak hour and Saturday peak hours to identify vehicle queuing needs. The 95th percentile queues were estimated using SimTraffic software, with results rounded to the nearest 25 feet to represent average vehicle lengths.

Because queues are based on an average of five traffic simulations using random arrivals, some fluctuation in results can be anticipated, particularly for movements that are near or projected to be over capacity.

Methodology

Available queue storage lengths were estimated using Google Earth Pro software and rounded to the nearest five feet. For turn lanes, two available storage values are stated: the first represents the striped storage; and the second is the effective storage, or the length physically available regardless of striping, such as a center turn lane upstream of a striped left-turn lane at an intersection. Although through lanes have no storage defined by striping, two values are reported for storage: the first is the distance to an upstream driveway; and the second is the distance to an upstream public street intersection.

Findings

The PM peak hour and Saturday 95th percentile queues are presented in Table . Bold text indicates the calculated queue exceeds the storage for the travel lane. SimTraffic output sheets are provided in Appendix I.



TABLE 5 – 95TH PERCENTILE QUEUING ANALYSIS								
		Available/	PM/Saturday Queue (feet)					
Intersection (Control)	Approach/		2024 Existing	2026 Pre- Development	2026 Post- Development			
Necarney City Road/	WBL	375/500	50/75	50/75	50/75			
Hwy 101 (Stop)	NB L+R	40/135	75/100	75/125	100/100			
Necarney City Road/	EB L+R	765/765	N/A	N/A	25/25			
Loop Road (Stop)	SB L+R	TBD	N/A	N/A	25/25			

As presented in Table 5, all existing and future conditions queues are expected to be accommodated by available storage. No queues will exceed available storage distances.



VII. MITIGATION AND RECOMMENDATIONS

All study area intersections are expected to operate at acceptable levels per ODOT and City standards with the addition of site trips, and vehicle queues will not exceed available storage.

The minimum required intersection sight distance of 280 feet is available from the driveways on Loop Road. The proposed intersection between Loop Road and Necarney City Road will address required sight distances through the design process.

Therefore, we do not recommend any mitigation measures for Necarney City Road or Loop Road.



VIII. APPENDIX

Appendix A. Figures

Appendix B. Scoping Material

Appendix C. Transit Information

Appendix D. Traffic Count Summaries

Appendix E. Seasonal Adjustment Calculations

Appendix F. In-Process Trips and Vicinity Map

Appendix G. Crash Data

Appendix H. Operations Calculations

Appendix I. Queuing Analysis

APPENDIX A.



Architecture - Interiors Planning - Engineering

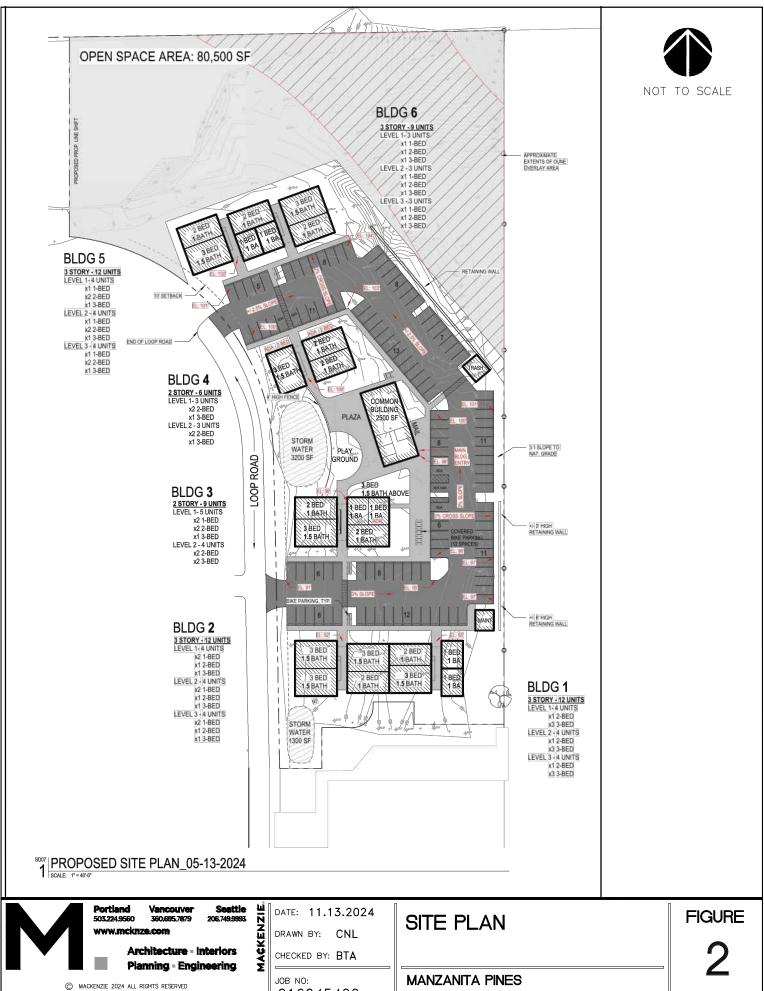
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JOB NO: 216045400 VICINITY MAP

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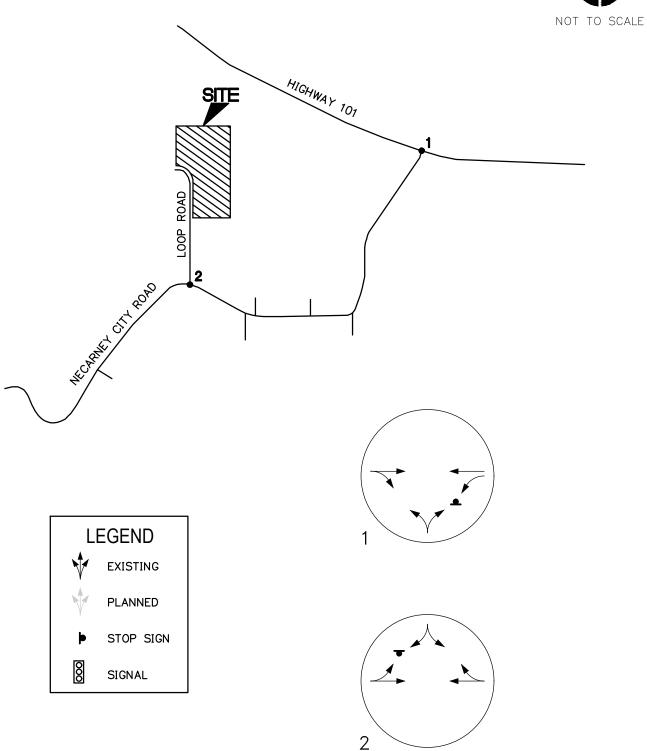


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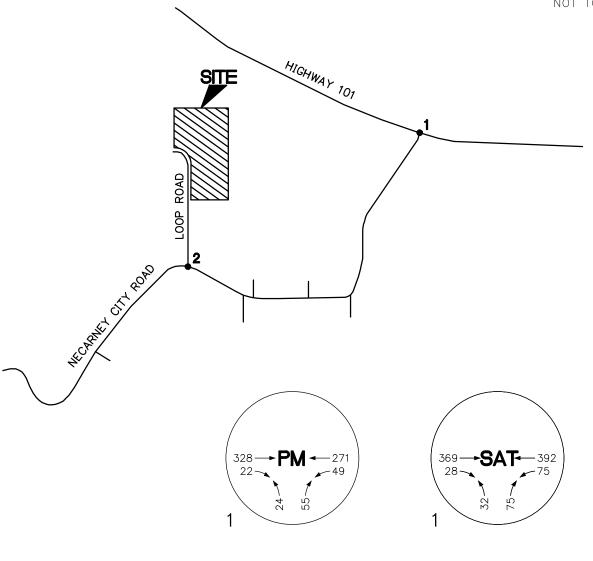
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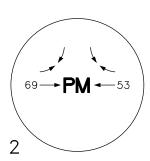
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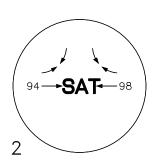
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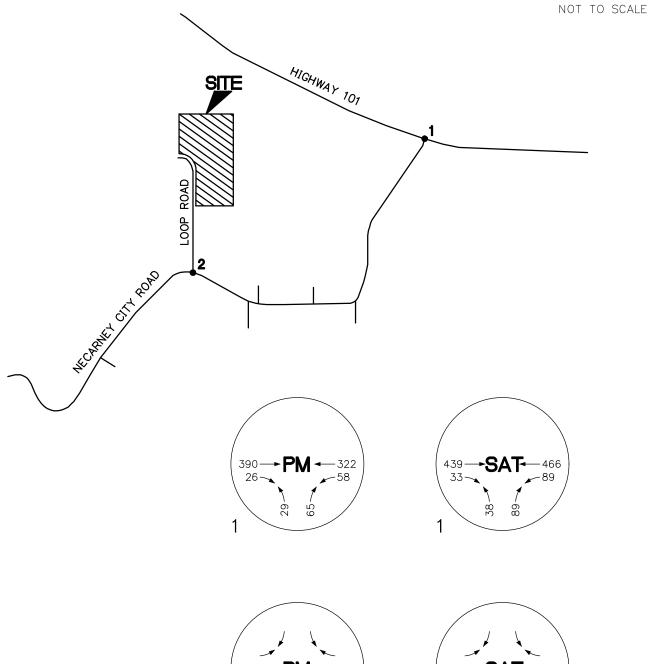
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2024 EXISTING TRAFFIC VOLUMES -PM + SATURDAY PEAK HOUR

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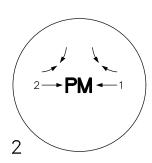
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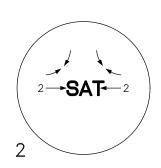
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HIGHWAY 101 SITE LOOP ROAD







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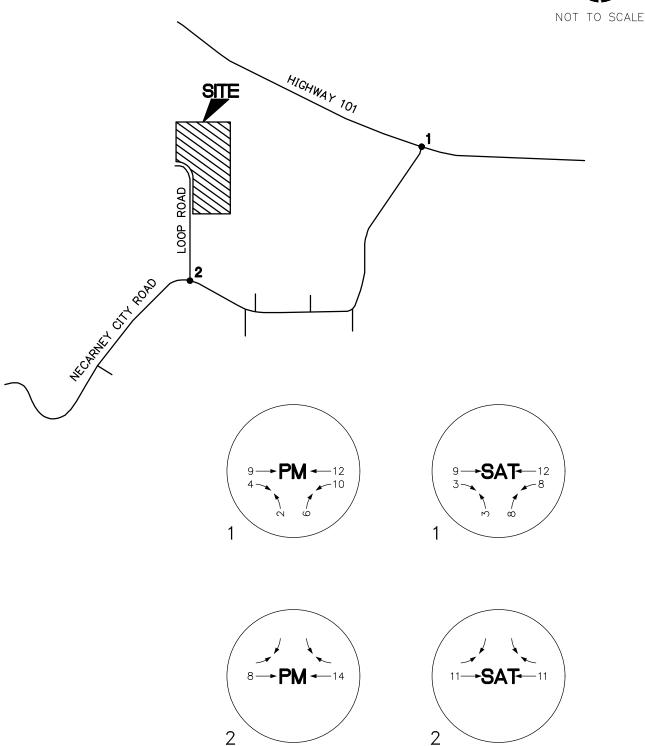
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JOB NO: 216045400 BACKGROUND TRAFFIC GROWTH, FIGURE 2 YEARS AT 1.0% PER YEAR -PM + SATURDAY PEAK HOUR

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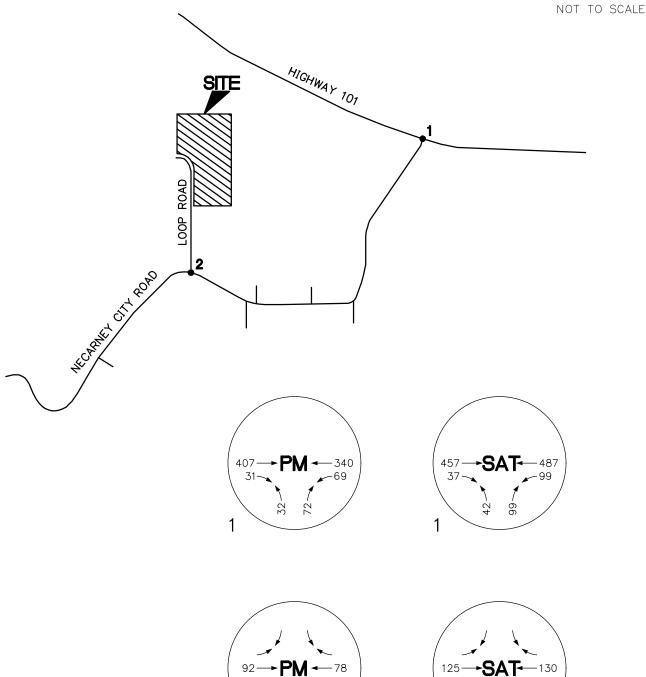
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JOB NO: 216045400 **IN-PROCESS** TRAFFIC VOLUMES -PM + SATURDAY PEAK HOUR

MANZANITA PINES MANZANITA, OREGON **FIGURE**

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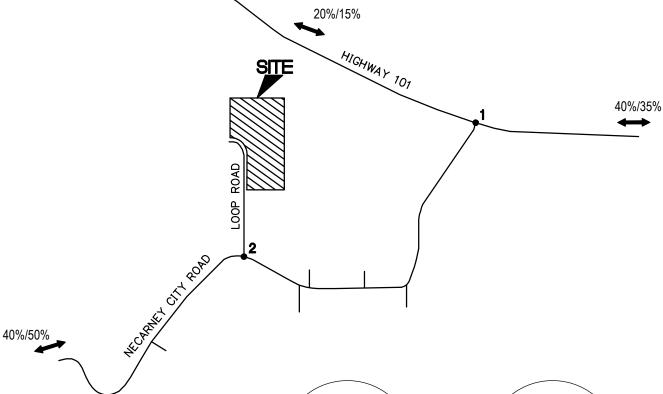
2026 PRE-DEVELOPMENT TRAFFIC VOLUMES -PM + SATURDAY PEAK HOUR

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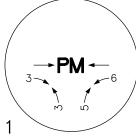


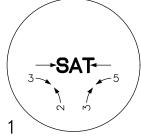
PM PEAK HOUR

Enter - 15

Exit - 13

Total - 28





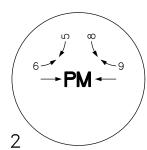
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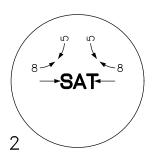
PEAK HOUR

Enter - 15

Exit - 10

Total - 25





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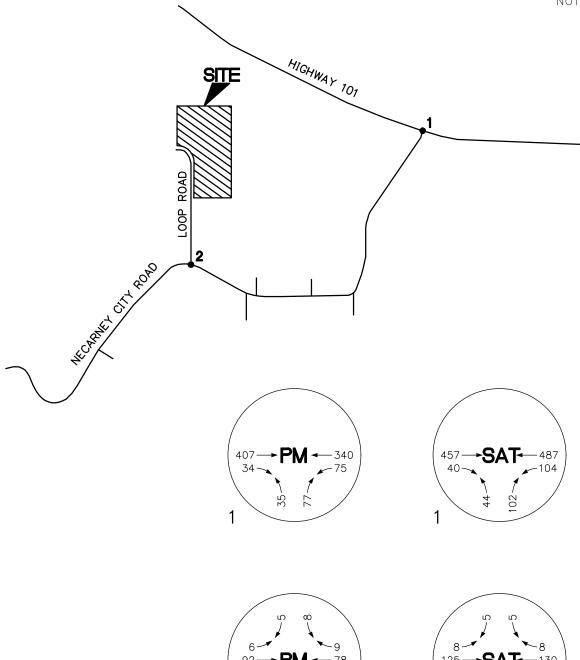
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PRIMARY TRIP DISTRIBUTION +
TRAFFIC ASSIGNMENT PM + SATURDAY PEAK HOUR

MANZANITA PINES MANZANITA, OREGON FIGURE

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2026 POST-DEVELOPMENT TRAFFIC VOLUMES -PM + SATURDAY PEAK HOUR

2

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APPENDIX B. **SCOPING MATERIAL**

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August 12, 2024

City of Manzanita Attention: Walt Wendolowski 167 S 5th Street Manzanita, OR 97130

Re: Oregon Coast Development Traffic Analysis

Traffic Impact Analysis Scope of Work

Project Number 2160454.11

Dear Mr. Wendolowski:

We have prepared this scoping assessment in response to your City Scoping letter dated May 23, 2024, for the affordable housing project to be constructed north of Necarney City Road along a new street, Loop Road. This proposed scope is more similar to recent traffic studies we prepared in the City.

STUDY AREA

The study area should be based on the trip impact at each intersection. To assess what impact is expected, we have prepared this assessment of trip generation and distribution. In general, ODOT requires analysis when impacts are 50 peak hour trips or more at an intersection, and some jurisdictions require analysis with impacts of 10 or more peak hour trips, unless there are known safety or capacity concerns.

We are providing trip generation and distribution estimates to determine the expected impact at each intersection recommended in the letter to be included in the study area.

Trip Generation

Trip estimates were developed based on the Institute of Transportation Engineer's Trip Generation Manual, 11th Edition for the affordable housing Land Use. Trip estimates for the proposed 60 units are 30 trips in the AM peak hour, 28 Trips in the weekday PM peak hour, and 289 daily as noted in Table 1.

Table 1 - Trip Generation									
ITE Codo	Cizo	AM Peak Hour		PM Peak Hour			Daily		
ITE Code	Land Use	Size	In	Out	Total	In	Out	Total	Daily
223	Affordable Housing (Income Limit)	60 DU	9	21	30	15	13	28	289

City of Manzanita Oregon Coast Development Traffic Analysis Project Number 2160454.11 August 12, 2024 Page 2

Trip Distribution

The following distribution of trips to the roadway network is proposed, based on similar studies for residential development and review of existing traffic volume patterns.

- 5% to/from Nehalem Bay State Park
- 20% to/from Central Manzanita
- 25% to/from the north on Highway 101
- 50% to/from the south on Highway 101

Trip Assignment

Based on the above generation and distribution of the project trips, Table 2 presents the estimated peak hour assignment at each of the intersections noted in the City's scoping letter.

Table 2 – Trip Assignment							
Intersection	Trip Distribution	Trip Assignment					
Necarney City Road/Highway 101	75%	23					
Necarney City Road/Loop Road	100%	30					
Necarney City Road/Meadows Drive	25%	8					
Necarney City Road/Classic Street	25%	8					
Highland Drive/Classic Street	20%	6					
Classic Street/Dorcas Lane	20%	6					
Classic Street/Laneda Avenue	20%	6					
Laneda Avenue/Highway101	25%	8					

Study Area Intersections

The following intersections are recommended for study based on the impact of 10 or more peak hour trips as noted in Table 2.

- Highway 101/Necarney City Road
- Necarney City Road/Loop Road

The intersection of Necarney City Road/Meadows Drive will only see an increase of up to eight trips, and no trips are expected to turn to or from Meadows Drive – only through trips on Necarney City Road.

The intersections on Classic Street at Highland Drive, Dorcas Lane, Laneda Avenue have all been reviewed by recent traffic studies and found to operate at acceptable levels and the addition of less than 10 peak hour trips is not expected to result in a significant change in operation.

City of Manzanita Oregon Coast Development Traffic Analysis Project Number 2160454.11 August 12, 2024 Page 3

TRAFFIC ANALYSIS SCOPE

The traffic analysis will follow City and ODOT standards and include the following elements.

Existing Traffic Counts

New counts will be conducted or obtained at the recommended study areas for the Weekday PM Peak Hour. This will involve acquiring and/or collecting turning movement count data for passenger vehicles, heavy vehicles, pedestrians, and bicycles to accurately understand the existing conditions.

Seasonal Adjustment per ODOT standards for Hwy 101 intersections.

A seasonal adjustment will be applied to traffic counts along Highway 101 as needed, depending on the date of the Counts. The ATR Characteristic Table Method and Seasonal Trend Method indicate that August is the peak time of the year for Highway 101.

Background Growth

Similar to recent studies prepared in Manzanita, we propose to apply a 1% growth rate per year for the study area intersections. Data from ODOT's 2040 Future Volumes Table from 0.2 miles north of Manzanita and 0.2 miles south of Laneda show less than 1% of growth, so 1% is a conservative estimate.

In-Process Trips

We are aware of the following projects which may need to be included as in-process with trips included in the predevelopment traffic volume estimates at the study area intersections.

- Manzanita Lofts 24 units off Dorcas Street
- Heron's Rest 26 units on S 3rd Street
- Nehalem Bay State Park expansion of existing facilities

Please confirm this list to be included and note if there are others recently approved.

Safety Review

We will present an evaluation of crashes at the study area intersections for the most recent five years of data available, and review sight distance availability in accordance with the AASHTO Policy on Geometric Design of Highways and Streets.

Analysis Methodology

This TIA will be prepared per *ODOT's Analysis and Procedures Manual, Version 2* and Synchro/SimTraffic software to analyze intersection operation and queuing.



City of Manzanita Oregon Coast Development Traffic Analysis Project Number 2160454.11 August 12, 2024 Page 4

Please let us know if you have any questions or comments on the proposed scope.

Sincerely,

Brent Ahrend, PE Associate Principal | Traffic Engineer

Enclosure(s): Attachment A – Site Plan

Attachment B – Trip Distribution Figure

c: Benjamin Pray – Home First Jim Pentz – Pine Grove Properties Inc Ralph Henderson – Mackenzie

Attachment A

SR/R ZONING 6.5 UNITS/AC BY RIGHT

13 UNITS/AC CLUSTERED (REQ. 40% OS)

PHASE I

APPROXIMATE EXTENTS OF DUNE

3:1 SLOPE TO NAT. GRADE

+/- 3' HIGH RETAINING WALL

+/- 6' HIGH **RETAINING WALL**

BLDG 1

3 STORY - 12 UNITS

x1 2-BED

x3 3-BED

x1 2-BED x3 3-BED LEVEL 3 - 4 UNITS x1 2-BED x3 3-BED

LEVEL 1-4 UNITS

LEVEL 2 - 4 UNITS

OVERLAY AREA

BLDG 6

3 STORY - 9 UNITS LEVEL 1-3 UNITS

x1 2-BED

x1 3-BED

x1 1-BED x1 2-BED x1 3-BED

x1 1-BED x1 2-BED x1 3-BED

RETAINING WALL

COVERED ---

____ BIKE PARKING__

EL: 96'

(12 SPACES)

LEVEL 2 - 3 UNITS

LEVEL 3 - 3 UNITS

PLAZA

PLAY GROUND

3 BED 1.5 BATH ABOVE

2 BED

1.BATH

3 BED.... 1.5 BATH

2 BED

1 BATH

2 BED

3 BED

1.5 BATH

1.BATH

STORM

WATER

2 BED

1.5 BATH

BIKE PARKING, TYP.

1.5 BATH

1.5 BATH

STORM

WATER

| 1300 SF

60 UNITS / 13 UNITS per acre = 4.6 ACRES REQUIRED (200,376sf) PROPOSED SITE AREA: 4.6ac (200,500sf)

40% OPEN SPACE = 1.8 ACRES (80,150 SF) PROPOSED OPEN SPACE: 80,500 SF

60 UNITS

1-BED (558) 7,812 2-BED (884)

> 53,812 TOTAL

120 PARKING SPACES

2:1.0 RATIO

COMMON BUILDING 2500 SF

DRIVE AISLE: 38,000 SF COMMON BLDG: BUILDINGS: PED PATHS: ACCESSORY STRUC:

77,200 SF

20,332 3-BED (1116) 25,668

IMPERVIOUS AREAS

2,500 SF 20,200 SF 13,500 SF 3,000 SF

TOTAL:

PROPOSED SITE PLAN_05-13-2024

OPEN SPACE AREA: 80,500 SF

10' SETBACK

BLDG 4

2 STORY - 6 UNITS LEVEL 1- 3 UNITS x2 2-BED x1 3-BED LEVEL 2 - 3 UNITS

BLDG 3

2 STORY - 9 UNITS LEVEL 1- 5 UNITS x2 1-BED

LEVEL 2 - 4 UNITS

BLDG 2

3 STORY - 12 UNITS LEVEL 1-4 UNITS

LEVEL 2 - 4 UNITS

LEVEL 3 - 4 UNITS

x2 1-BED

x1 2-BED x1 3-BED

x2 1-BED x1 2-BED

x1 3-BED

x2 1-BED

x1 2-BED

x1 3-BED

x2 2-BED x1 3-BED

x2 2-BED

x2 3-BED

x2 2-BED x1 3-BED

END OF LOOP ROAD

BLDG 5

3 STORY - 12 UNITS LEVEL 1- 4 UNITS

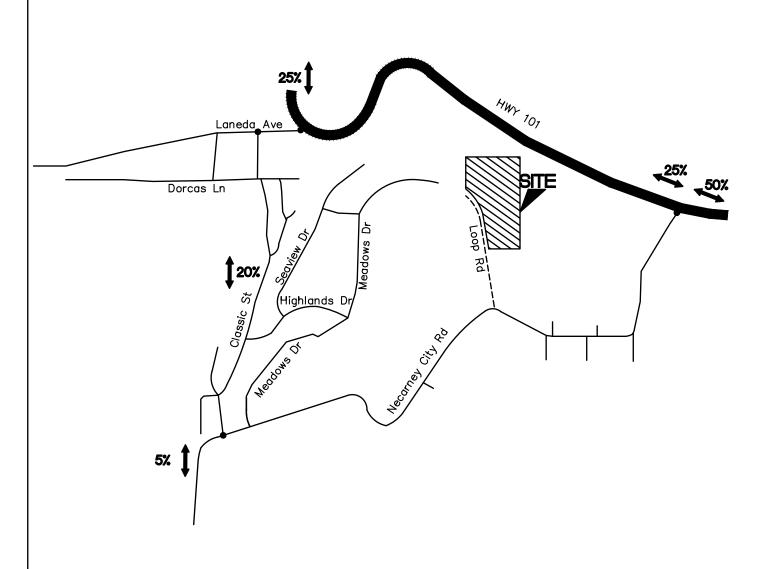
x1 1-BED x2 2-BED x1 3-BED LEVEL 2 - 4 UNITS

x1 3-BED LEVEL 3 - 4 UNITS x1 1-BED x2 2-BED x1 3-BED

x1 1-BED x2 2-BED

SD07

SITE PLAN





Portland 503,224,9560

Architecture - Interiors Planning - Engineering

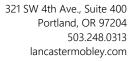
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MACKENZIE DATE: 08.01.2024

DRAWN BY: FMS CHECKED BY: BTA

JOB NO: 2160454.11 TRIP DISTRIBUTION

OREGON COAST DEVELOPMENT MANZANITA, OR





October 2, 2024

Scott Gebhart City of Manzanita 543 Laneda Avenue Manzanita, OR 97130

Dear Scott,

At your request, I have reviewed the development plan for the proposed workforce housing on the 12.54 Pine Grove Properties site that was recently annexed into the City of Manzanita. I understand that the applicant is proposing the first of two development phases, with the first phase being 60 dwelling units and the second phase being 68 dwelling units, for a total of 128 units at buildout.

Transportation Impact Study

It is recommended that a Transportation Impact Study (TIS) be conducted and submitted as part of the land use application. This letter provides a detailed scope of work for the applicant. The TIS should be prepared by a professional engineer registered in Oregon with specific experience in transportation engineering.

Trip Generation & Distribution

Project-generated trips should be calculated based on the 11th Edition of the *Trip Generation Manual*, published by the Institute of Transportation Engineers (ITE). If other trip generation rates or information are used, they should first be reviewed and approved by the City of Manzanita.

The distribution of project-generated trips should be assigned to the surrounding roadway network based on the traffic count data as well as anticipated trip origins and destinations and expected travel routes to and from the site. Access to the site will be only via Necarney City Road on the eastern edge of the Manzanita city limits. The TIS should quantify the number of trips that will travel to and from the east toward Highway 101 and the number of trips that will travel to and from the west to Manzanita and Highway 101 at Laneda Avenue. Local destinations in and near Manzanita should also be identified.

Project Study Area

Based on the anticipated trip generation and distribution, traffic counts and a full operational analysis shall be required at the intersection of Necarney City Road and the new site access location. Traffic counts shall be conducted at these intersections during typical weekday conditions during the evening peak hours (4:00 to 6:00 PM) as well as the Saturday afternoon peak (noon to 3:00 PM).

The operational analysis of the study-area intersection shall include left-turn lane warrants to determine the potential need for an eastbound left-turn lane on Necarney City Road, as well as an examination of sight distance. Requirements for intersection and stopping sight distances shall be based on the standards in the 7th Edition of *A Policy on Geometric Design of Highways and Streets*, published by AASHTO.

It is recognized that only the first phase of development is proposed at this time, but it is recommended that the applicant examine conditions with the site at full build out to ensure that the new intersection is constructed in a manner that can accommodate the long-term demands of the site. This will help avoid future modifications to the intersection.

If you have any questions regarding this scope of work, please do not hesitate to call.

Sincerely,

Todd E. Mobley, PE

Principal



Clara Layton

From: Todd Mobley <todd@lancastermobley.com>

Sent: Tuesday, October 29, 2024 10:56 AM

To: Clara Layton

Cc: Brent Ahrend; Scott Gebhart

Subject: Re: Manzanita Workforce Housing TIA Scoping Letter

Attachments: Traffic Counts - 45411.pdf; 11LTR-City of Manzanita-Traffic Scoping-240812.pdf

Follow Up Flag: Follow up Flag Status: Completed

Hi Clara,

I have inserted my comments below in red. Thanks for sending this along and let me know if you have any questions.

-Todd

Todd E. Mobley, PE

Principal



1130 SW Morrison St, Suite 318 | Portland, OR 97205

P: 503-248-0313 C: 503-319-9811 Website: <u>lancastermobley.com</u>

Offices: Portland, OR | Bend, OR | Vancouver, WA

On Thu, Oct 24, 2024 at 3:46 PM Clara Layton < CLayton@mcknze.com > wrote:

Good afternoon!

I'm forwarding on Brent's scoping letter, and a few questions:

Our traffic counter equipment was vandalized and we have counts until 2:45 PM. Will that work?
 Counts attached.

It looks like you captured the peak, so I think that should be fine.

• Your scoping letter didn't include mention of any in-process projects. Can you confirm that we should include the following: Manzanita Lofts, Heron's Rest, and possible trips from the expansion in Nehalem Bay State Park? Do you have any further information about the expansion?

Your list is accurate, but there have also been some other smaller projects that didn't do traffic studies, so I would suggest a growth rate in addition to the in-process trips. The 1% you suggest below seems reasonable for this. As for the State Park, their master plan was just approved last month by Tillamook County. My understanding is that the immediate projects at the park will be maintenance and not expansion, but the master plan does include new campsites, more parking at the marina, and associated park upgrades. ODOT told Parks that they wouldn't generate more than 50 peak hour or 500 daily trips so they didn't need a traffic study. Arielle in Region 2 Traffic provided some trip generation info and helped them respond to some opposition testimony. That information is here:

https://www.tillabook.org/sites/default/files/fileattachments/community_development/project/95710/materials_provided_by_applicant_at_july_11_2024_hearing_nehalem_bay_state_park.pdf

I would recommend adding trips for the park expansion as in-process.

• Can you approve a growth rate of 1% per year? We're calculating a seasonal adjustment factor based on Coastal Destination.

This is acceptable.

• We had Necarney City Rd/Pine Ridge Ln counted to determine trip distribution as the closest intersection with a comparable development. The count is helpful for roadway volumes, but we're finding the gated community trip distribution inconsistent with our assumptions. We'll keep crunching the numbers, let us know what you think.

The count data you have at Pine Ridge shows something close to a 60/40 split with the majority out Necarney to 101. Google shows the fastest route from the site to points along 101, even points to the north, is via Necarney. That might be, but it is probably more dependent on the perception of local drivers about where it is easier to turn left onto the highway. A gated community might have a different distribution than workforce housing, but I would expect the workforce housing might have a heavier split into Manzanita than the Pine Ridge neighborhood since most local employment would be in Manzanita proper.

I'll let you and Brent sort out the analysis, but those are my thoughts on the distribution. You might also say a few things in the TIA about sensitivity because I suspect small changes to the distribution percentages won't give you different results and findings overall.

Clara Layton EIT

D 971-254-9496

Transportation Planning Professional Licenses & Certifications



Mackenzie.

APPENDIX C.

TRANSIT INFORMATION

Fares/ Tarifas

Each	Way, Per Zone/	
lda o	vuelta, por zona	.\$1.50

Zone 1: Hobsonville Point (S. of Garibaldi) to Sand Lake Rd (N. of Hemlock)

Zone 2: Clatsop County Line to Hobsonville Point (S. of Garibladi)

Zone 3: Sand Lake Rd (N. of Hemlock) to Lincoln County Line

<u>Lincoln County Zone:</u> Starts at Lincoln County Line

Clatsop County Zone: Starts at Clatsop County Line

Child Fares/ Tarifas Para Niños

First Child/ Primer Niño (0-4).....FREE Additional Child/ Niño adicional (0-4)...1/2 Fare Child/ Niño (5-11)......1/2 Fare (When traveling with a full fare adult/ Al viajar con un adulto que paga la tarifa completa)

Monthly Pass/ Pase de Un Mes

Regular/ Regula	ır	\$40
Reduced/ Descu	uento	\$30

Reduced fares offered for age 60+, children, & individuals with verifiable short or long term disability/ Se ofrecen tarifas con descuento para mayores de 60 años, niños y personas con discapacidades de corto o largo plazo comprobables

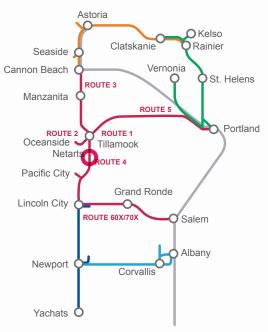
No Bus Service/ No Hay Servicio de Autobuses

New Years Day/ Año Nuevo Thanksgiving Day/ Día de Acción de Gracias Christmas Day/ Navidad

Route & Schedule Info/ Información de Rutas y Horarios

800-815-8283 www.TillamookBus.com 800-735-2700/TTY

NWCONNECTOR NWCONNECTOR.ORG



NWCONNECTOR Visitor Pass/ Pase

Para Visitantes
3 Days/ 3 Dias \$25

3 Days/ 3 Días \$25 7 Days/ 7 Días \$30

(includes a round trip to Portland or Salem and unlimited travel on NWConnector routes/ Incluye un viaje redondo a Portland o Salem y viajes ilimitados en las rutas de NWConnector)

CONNECTING SERVICES/ SERVICIOS DE CONEXIÓN

Lincoln County Transit

nwconnector.org | 541-265-4900

Sunset Empire Transportation District

nwconnector.org | 503-861-7433

Point Bus

oregon-point.com | 1-888-846-4183

Greyhound

greyhound.com | 1-800-231-2222

Amtrak

amtrak.com | 1-800-872-7245

Tri-Met

trimet.org | 503-238-7433

ROUTE/ RUTA 3

Tillamook - Cannon Beach

Effective January 23, 2022 A partir del 23 de enero de 2022



Tillamook County
Transportation District



Tillamook Transit Center

SERVICE OPERATES 7 DAYS A WEEK EL SERVICIO OPERA LOS 7 DÍAS DE LA SEMANA







FOR REAL TIME BUS INFO, DOWNLOAD THE TRANSIT APP TODAY!/
PARA OBTENER INFORMACIÓN SOBRE LOS AUTOBUSES EN
TIEMPO REAL, DESCARGUE LA APLICACIÓN TRANSIT.

Transit Center 2nd & Laurel	Tillamook Fred Meyer	s Idaville	P Bay City	9 Garibaldi	9 Rockaway Beach	7 Wheeler	8 Nehalem	6 Manzanita	Cannon Beach
Northbo	ound								
4:55	5:00	5:06	5:09	5:17	5:27	5:45	5:53	5:59	
9:03	9:08	9:14	9:17	9:25	9:35	9:53	10:01	10:07	10:27
1:50	1:55	2:01	2:04	2:12	2:22	2:40	2:48	2:54	3:14
6:05	6:10	6:16	6:19	6:27	6:37	6:55	7:03	7:09	7:29

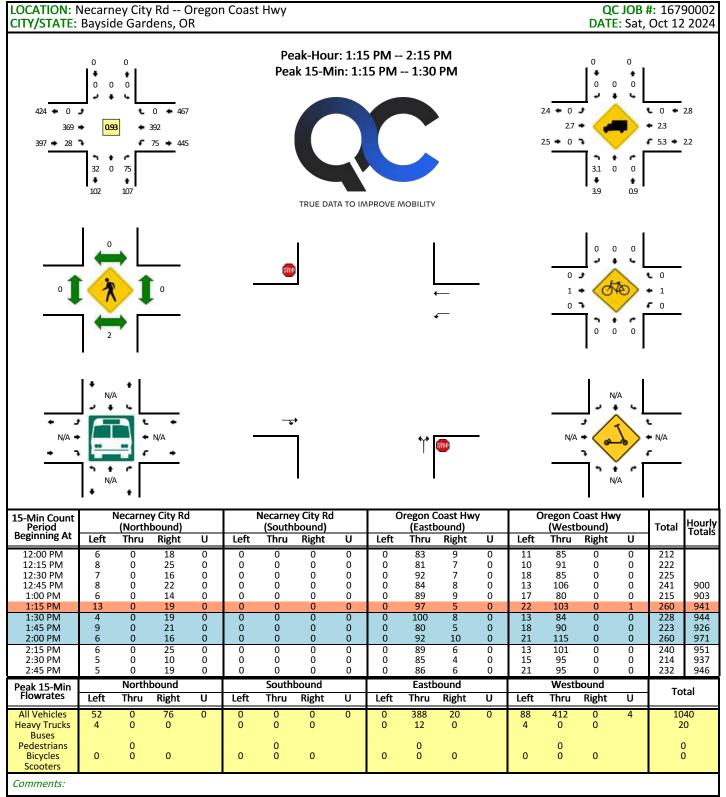
Bold/ Negritas = PM

Cannon Beach	6 Manzanita	8 Nehalem	Wheeler 7	9 Rockaway Beach	5 Garibaldi	b Bay City	8 Idaville	Tillamook Fred Meyer	Transit Center 2nd & Laurel
Southbo	ound								
	6:09	6:15	6:23	6:41	6:51	6:59	7:02	7:08	7:13
10:37	10:57	11:03	11:11	11:29	11:39	11:47	11:50	11:56	12:01
3:24	3:44	3:50	3:58	4:16	4:26	4:34	4:37	4:43	4:48
7:39	7:59	8:05	8:13	8:31	8:41	8:49	8:52	8:58	9:03

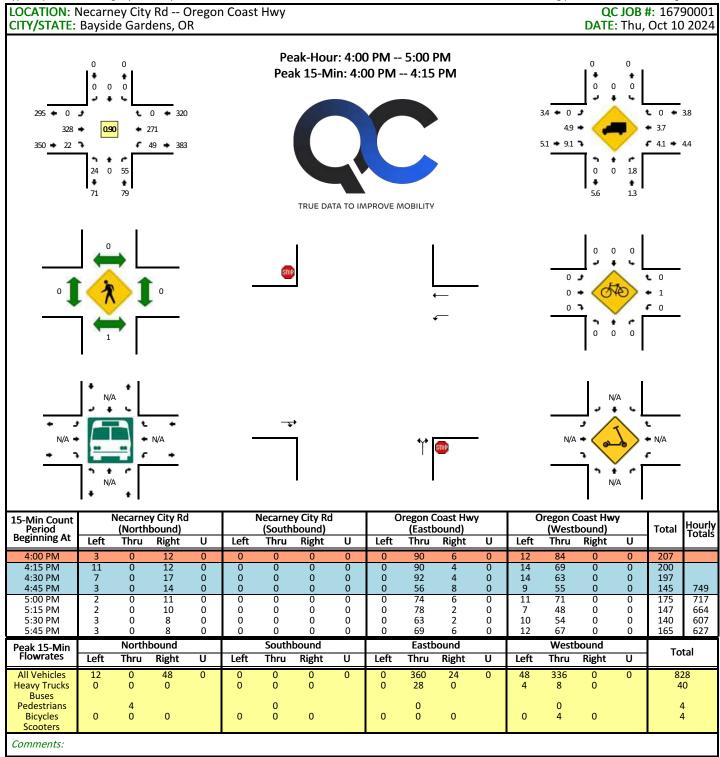
Bold/ Negritas = PM

APPENDIX D.

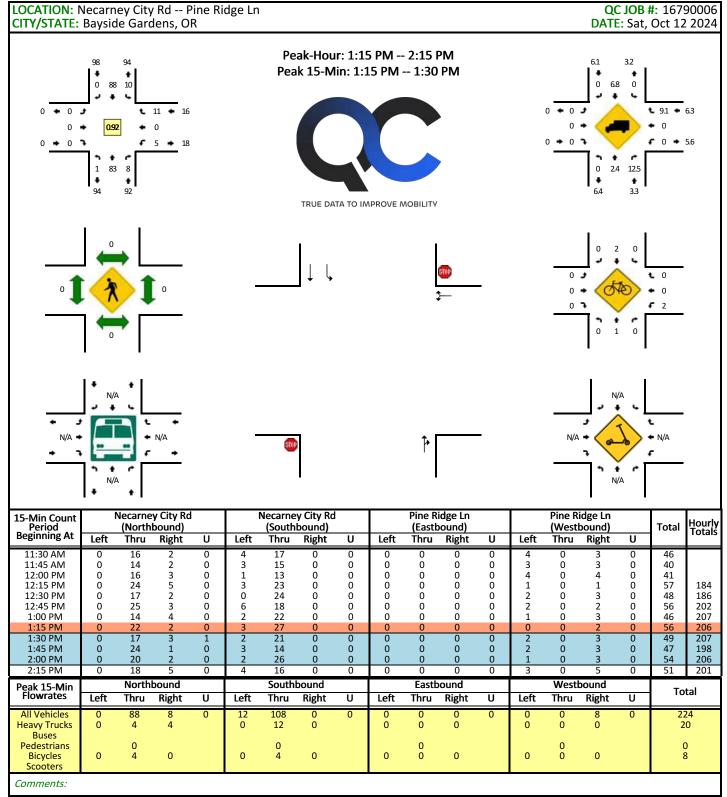
TRAFFIC COUNT SUMMARIES



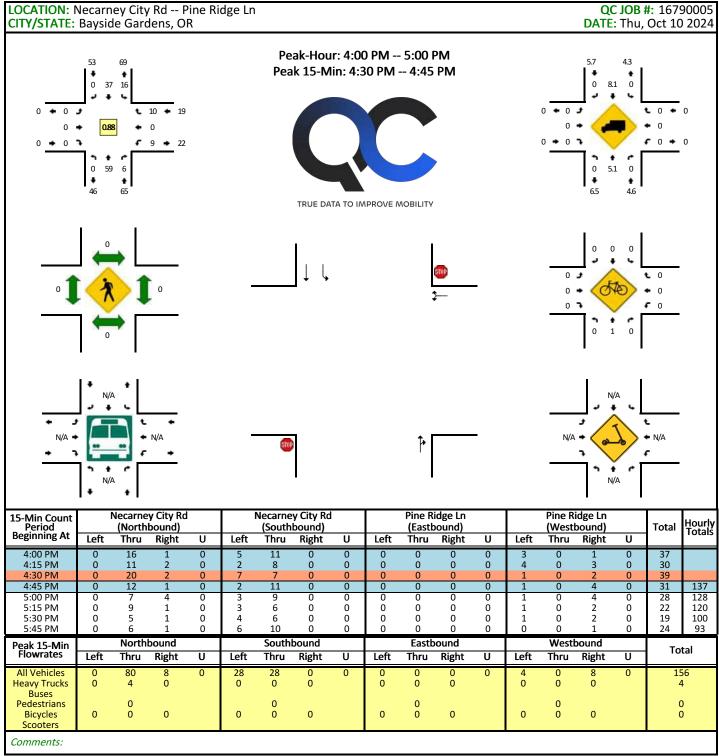
Report generated on 10/24/2024 8:41 AM



Report generated on 10/24/2024 8:40 AM



Report generated on 10/24/2024 8:42 AM



Report generated on 10/24/2024 8:40 AM

APPENDIX E.

SEASONAL ADJUSTMENT CALCULATIONS

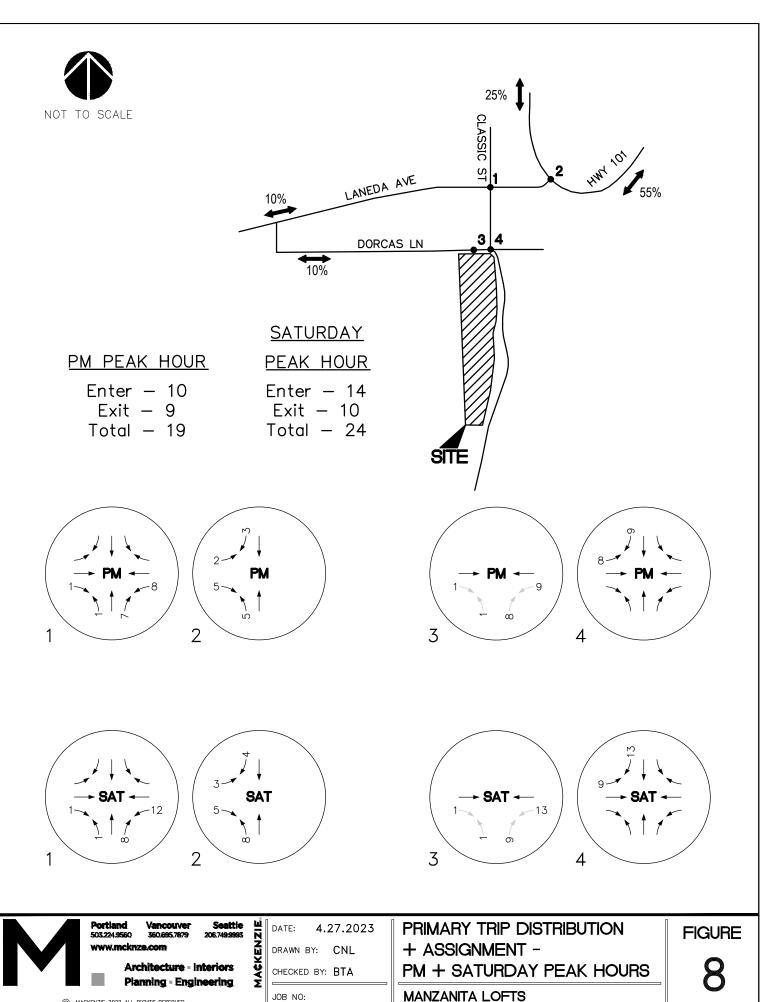
							S	EASONA	L TREN	TABLE	(Updated	d: 11/08/2	023)												Seasonal Trend
TREND	1-Jan	15-Jan	1-Feb	15-Feb	1-Mar	15-Mar	1-Apr	15-Apr	1-May	15-May	1-Jun	15-Jun	1-Jul	15-Jul	1-Aug	15-Aug	1-Sep	15-Sep	1-Oct	15-Oct	1-Nov	15-Nov	1-Dec	15-Dec	Peak Period Factor
INTERSTATE URBANIZED	1.0869	1.1041	1.0688	1.0335	1.0182	1.0028	0.9995	0.9962	0.9901	0.9840	0.9641	0.9443	0.9502	0.9562	0.9510	0.9458	0.9575	0.9692	0.9791	0.9891	1.0107	1.0324	1.0532	1.0739	0.9443
INTERSTATE NONURBANIZED	1.2459	1.2915	1.2286	1.1657	1.0907	1.0158	1.0059	0.9960	0.9728	0.9496	0.9128	0.8760	0.8650	0.8540	0.8612	0.8684	0.8905	0.9126	0.9488	0.9850	1.0336	1.0822	1.1717	1.2612	0.8540
COMMUTER	1.0905	1.0986	1.0636	1.0285	1.0162	1.0038	0.9959	0.9879	0.9814	0.9749	0.9631	0.9512	0.9614	0.9717	0.9608	0.9500	0.9548	0.9595	0.9634	0.9673	1.0090	1.0507	1.0733	1.0958	0.9500
COASTAL DESTINATION	1.2064	1.1715	1.1234	1.0753	1.0545	1.0337	1.0372	1.0407	1.0216	1.0024	0.9586	0.9147	0.8760	0.8372	0.8371	0.8370	0.8678	0.8985	0.9578	1.0170	1.0730	1.1290	1.1823	1.2357	0.8370
COASTAL DESTINATION ROUTE	1.3937	1.2897	1.2245	1.1594	1.1247	1.0901	1.0911	1.0921	1.0516	1.0111	0.9493	0.8875	0.8172	0.7469	0.7455	0.7440	0.7916	0.8391	0.9274	1.0158	1.1126	1.2094	1.3193	1.4291	0.7440
AGRICULTURE	1.4537	1.4624	1.3705	1.2786	1.2139	1.1492	1.1207	1.0923	1.0075	0.9226	0.8742	0.8258	0.8348	0.8439	0.8422	0.8405	0.7976	0.7547	0.8073	0.8598	1.0041	1.1484	1.3339	1.5194	0.7547
RECREATIONAL SUMMER	1.6049	1.5814	1.4924	1.4034	1.3208	1.2382	1.2380	1.2377	1.0939	0.9500	0.8669	0.7839	0.7392	0.6945	0.7065	0.7185	0.7404	0.7624	0.8468	0.9311	1.1270	1.3230	1.5054	1.6879	0.6945
RECREATIONAL SUMMER WINTER	1.0075	0.9570	0.9184	0.8799	0.9701	1.0603	1.0675	1.0747	1.0843	1.0939	1.0045	0.9151	0.8244	0.7336	0.7795	0.8254	0.9368	1.0482	1.1794	1.3105	1.4969	1.6833	1.3470	1.0108	0.7336
RECREATIONAL WINTER**	0.8059	0.6710	0.6475	0.6240	0.7462	0.8685	0.9307	0.9928	1.1496	1.3064	1.2173	1.1282	0.9996	0.8709	0.9526	1.0342	1.1225	1.2108	1.4061	1.6013	1.9826	2.3639	1.6332	0.9026	0.6240
SUMMER	1.2374	1.2352	1.1733	1.1114	1.0786	1.0459	1.0330	1.0202	0.9851	0.9500	0.9160	0.8819	0.8660	0.8501	0.8561	0.8620	0.8891	0.9161	0.9430	0.9698	1.0525	1.1352	1.2002	1.2653	0.8501
SUMMER < 2500	1.2836	1.2576	1.1943	1.1310	1.1011	1.0712	1.0448	1.0184	0.9633	0.9082	0.8861	0.8641	0.8609	0.8578	0.8695	0.8813	0.8874	0.8936	0.9165	0.9394	1.0500	1.1607	1.2535	1.3463	0.8578

^{*} Seasonal Trend Table factors are based on previous year ATR data. The table is updated yearly.
* Grey shading indicates months were seasonal factor is greater than or less than 30%

Seasonal Adjustment Factor (October 10th): 1.19

^{**}Use Recreation Winter Trend with Caution! ATR site was down for most of of 2022 due to loop issues and was estimated while the site was down

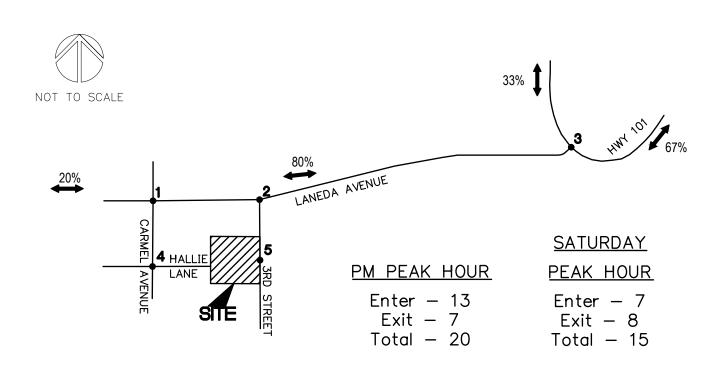
APPENDIX F. **IN-PROCESS TRIPS**

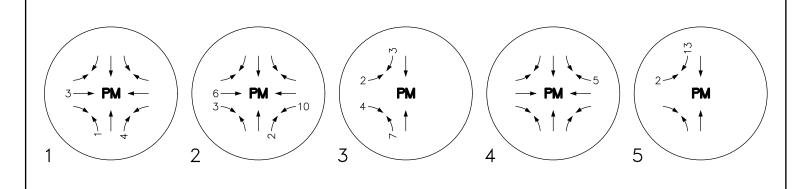


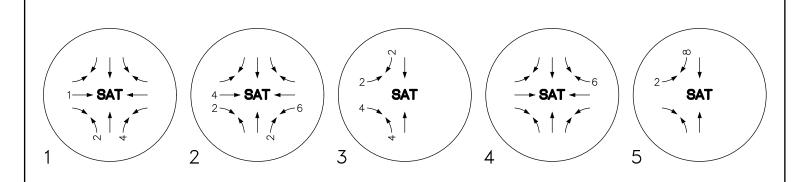
222012000

MANZANITA, OREGON

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Portland

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MACKENZIE Architecture - Interiors Planning - Engineering

9.21.2022 DATE:

DRAWN BY: CNL

CHECKED BY: JTJ

JOB NO: 222019400 PRIMARY TRIP DISTRIBUTION + ASSIGNMENT -

HERON'S REST MANZANITA, OREGON **FIGURE**

ODOT REVISED Trip Generation Analysis

Received from Arielle Childress by email 7.11.2024, 3:21 PM

							Rates				Т	otal Trip	os				In/Ou	ıt Trips		
					- 1	Weekday	1	Wee	kend	V	Veekda	у	Wee	kend		Wee	kday		Week	kend
ITE Code	Land Use Description	Independent Variable	No. of Units	Avg Rate or Eq	Daily Rate	AM Peak Rate	PM Peak Rate	Daily Rate	Peak Rate	Daily Trips	AM Peak Trips		Daily Trips	Peak Trips	AM Trips In	AM Trips Out	PM Trips In	PM Trips Out	Peak Trips In	Peak Trips Out
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416	Campground/RV Park	Acre(s)	21	Avg		0.48	0.98	ng out	Totals		10	21			4	6	14	7		
420	Marina	Berth(s)	32	Avg	2.41	0.07	0.21	2.61	0.22	78	2	7	84	7	1	1	4	3	3	4
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Eby.																				
9, 1			San Paris					sed Use		78	12									

JOHNSON Tracy * OPRD

From:

CHILDRESS Arielle < Arielle. CHILDRESS@odot.oregon.gov>

Sent:

Thursday, July 11, 2024 3:20 PM

To:

JOHNSON Tracy * OPRD

Subject:

RE: Nehalem Bay State Park Improvements ITE Codes

Here's the table as a picture. If it doesn't print properly from within the email you should be able to easily place into a word doc.

						Rates				Т	otal Trip
				١	Veekday	,	Wee	kend	٧	Veekda	У
ITE Cade Land Use Description	Independent Variable	No. of Units	Avg Rate or Eq	Daily Rate	AM Peak Rate	PM Peak Rate	Daily Rate	Peak Rate	Daily Trips	AM Peak Trips	PM Peak Trips
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416 Campground/RV Park	Acre(s)	21	Avg		0.48	0.98				10	21
420 Marina	Berth(s)	32	Avg	2.41	0.07	0.21	2.61	0.22	78	2	7
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		THE PARTY									
						Propo	sed Us	e Totals	78	12	28

Arielle Childress, P.E. (she/her/hers)

Traffic Analysis Engineer

ODOT Region 2 455 Airport Rd. SE, Bldg. B, Salem, OR 97031 (971) 208-1290

From: JOHNSON Tracy * OPRD < Tracy. JOHNSON@oprd.oregon.gov>

Sent: Thursday, July 11, 2024 3:13 PM

To: CHILDRESS Arielle <Arielle.CHILDRESS@odot.oregon.gov>
Subject: RE: Nehalem Bay State Park Improvements ITE Codes

You don't often get email from tracy.johnson@oprd.oregon.gov. Learn why this is important

This message was sent from outside the organization. Treat attachments, links and requests with caution. Be conscious of the information you share if you respond.

Hi again,

I have a huge favor to ask. Could you please pdf the table and send it as an attachment? It isn't printing correctly within the body of the email text.

Thanks,



Tracy Johnson, PLA | Senior Project Manager

OPRD | Central Park Services – Park Improvement, Engineering Division 971.283.6805

From: CHILDRESS Arielle < Arielle. CHILDRESS@odot.oregon.gov >

Sent: Thursday, July 11, 2024 2:47 PM

To: Cassandra Dobson < CDobson@parametrix.com >; VYMAZAL Zdenek G < Zdenek.G.VYMAZAL@odot.oregon.gov >

Cc: JOHNSON Tracy * OPRD < Tracy.JOHNSON@oprd.oregon.gov >; Jennifer Hughes < JHughes@parametrix.com >; Ryan Rudnick < RRudnick@parametrix.com >

Subject: RE: Nehalem Bay State Park Improvements ITE Codes

You don't often get email from arielle.childress@odot.oregon.gov. Learn why this is important

Cassandra,

Please see responses below in red. A note on LUC 411 (Public Park), it doesn't provide daily weekday trips when using Dail Trail Users as the independent variable. It only provides data for Saturday and Sunday. It also only has one data point so I caution against using it. Therefore, I applied LUC 420 (Marina) instead.

- Are the empty rows for total trips and in/out trips for campground/RV park (highlighted below) intentionally empty, or is there data that should be shown there? No daily weekday data is available for this land use. A very rough way to estimate is assume the PM peak hour is 10% of daily traffic, but this is a very general traffic assumption and not specific to your land use. This translates to the Campground/RV park having 210 daily trips with the total proposed uses having 288 daily trips. If it's absolutely necessary you can modify the table below as I embedded it as a table, and not a picture.
- Is it possible to show the total trips from the proposed improvements as "proposed use totals" rather than "existing use totals" to make it clear that they are trips resulting from the proposed changes? I've moved the trip generation down into the "proposed use" section.
- It appears that the analysis included both Marina (420) and Public Park (411) codes to analyze trips from the new boat ramp parking spaces is it possible to revise the analysis to include one or the other? As it stands now, both are included in the total trips measurements, which means we are accounting for the new parking lot traffic twice in the calculations. Please see the request from the original email copied below we were hoping to provide two alternative codes to measure the trips from the parking lot based on which ODOT felt was most appropriate, rather than adding the trips from both methods. LUC 420 (Marina) provides better data than the public park for the variables provided, therefore I'm going to just apply LUC 420

							Rates				T	otal Trip
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ITE Code	Land Use Description	Independent Variable	No. of Units	Avg Rate or Eq	Daily Rate	AM Peak Rate	PM Peak Rate	Daily Rate	Peak Rate	Daily Trips	AM Peak Trips	PM Peak Trips
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SHAR												
			146000									
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416	Campground/RV Park	Acre(s)	21	Avg		0.48	0.98	ng Use	lotais		10	21
420	Marina	Berth(s)	32	Avg	2.41	0.07	0.21	2.61	0.22	78	2	7
12/0955			E.M.									
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10 A 1/1			L STATE OF S						_			
W/Line												

Please let me know if you need anything else. I only work until 3:30 PM if you need a response back by today.

Thanks!

Arielle Childress, P.E. (she/her/hers)

Traffic Analysis Engineer ODOT Region 2 455 Airport Rd. SE, Bldg. B, Salem, OR 97031 (971) 208-1290

From: Cassandra Dobson < CDobson@parametrix.com >

Sent: Thursday, July 11, 2024 1:56 PM

To: VYMAZAL Zdenek G <Zdenek.G.VYMAZAL@odot.oregon.gov>

Cc: JOHNSON Tracy * OPRD < Tracy.JOHNSON@oprd.oregon.gov>; Jennifer Hughes < JHughes@parametrix.com>; Ryan Rudnick < RRudnick@parametrix.com>;

CHILDRESS Arielle < Arielle. CHILDRESS@odot.oregon.gov >

Subject: RE: Nehalem Bay State Park Improvements ITE Codes

This message was sent from outside the organization. Treat attachments, links and requests with caution. Be conscious of the information you share if you respond.

Thank you Z! If the traffic team is able to answer our questions today, we would greatly appreciate it.

From: VYMAZAL Zdenek G <Zdenek.G.VYMAZAL@odot.oregon.gov>

Sent: Thursday, July 11, 2024 1:50 PM

To: Cassandra Dobson < CDobson@parametrix.com>

Cc: JOHNSON Tracy * OPRD < tracy.johnson@oprd.oregon.gov >; Jennifer Hughes < JHughes@parametrix.com >; Ryan Rudnick < RRudnick@parametrix.com >;

CHILDRESS Arielle < Arielle. CHILDRESS@odot.oregon.gov>

Subject: RE: Nehalem Bay State Park Improvements ITE Codes

Hello Cassandra,

I know it is late. I took time of recently and was very busy after that.

I have forwarded your questions to the traffic people. Who could answer your questions better than me.

Thank you

Z

Zdenek "Z" Vymazal, PE, PLS Development Review Coordinator (Area 1) ODOT – Region 2 455 Airport Rd. SE, Bldg. B Salem, OR 97301 (971)-345-1318 Cell/Office

zdenek.g.vymazal@odot.oregon.gov

Hours: 6:30 AM to 3:00 PM Monday - Friday

From: Cassandra Dobson < CDobson@parametrix.com>

Sent: Thursday, July 11, 2024 1:13 PM

To: VYMAZAL Zdenek G < Zdenek.G. VYMAZAL@odot.oregon.gov>

Cc: JOHNSON Tracy * OPRD < Tracy.JOHNSON@oprd.oregon.gov >; Jennifer Hughes < JHughes@parametrix.com >; Ryan Rudnick < RRudnick@parametrix.com >

Subject: RE: Nehalem Bay State Park Improvements ITE Codes

Importance: High

This message was sent from outside the organization. Treat attachments, links and requests with caution. Be conscious of the information you share if you respond.

Hello Z,

I just left you a voicemail, but wanted to follow up again on the below request. Is it possible to receive a response to our questions below before tonight's public hearing so that we may submit them to the record?

Thank you so much for your time, please let me know if you have any questions.

Cass

From: Cassandra Dobson

Sent: Tuesday, July 9, 2024 8:55 AM

To: VYMAZAL Zdenek G < Zdenek.G. VYMAZAL@odot.oregon.gov >

Cc: JOHNSON Tracy * OPRD < tracy.johnson@oprd.oregon.gov >; Jennifer Hughes < JHughes@parametrix.com >; Ryan Rudnick < RRudnick@parametrix.com >

Subject: RE: Nehalem Bay State Park Improvements ITE Codes

Hello Z,

I wanted to reach out to follow up on the below request. Please let us know if you have any questions or need any additional information at this time.

Thank you!

Cass

From: Cassandra Dobson

Sent: Tuesday, July 2, 2024 4:20 PM

To: VYMAZAL Zdenek G < Zdenek.G. VYMAZAL@odot.oregon.gov>

Cc: JOHNSON Tracy * OPRD < tracy.johnson@oprd.oregon.gov >; Jennifer Hughes < JHughes@parametrix.com >; Ryan Rudnick < RRudnick@parametrix.com >

Subject: RE: Nehalem Bay State Park Improvements ITE Codes

Good afternoon Z,

Thank you again for taking the time earlier this year to review potential traffic impacts from our proposed improvements at Nehalem Bay State Park. We have submitted our application for a Conditional Use Master Plan to Tillamook County and will have a public hearing with their Planning Commission next Thursday. During the application review, we have received some questions on ODOT's analysis (below) and are hoping that you may be able to help us address them. If at all possible, it would be wonderful if we could get responses to these questions and any necessary revisions to the analysis no later than **end of day, Wednesday, July 10**th so that we may incorporate them into our presentation at the Planning Commission meeting on the 11th. I know we have a short week this week due to the holiday, so we are very appreciative of any assistance you can provide in that time.

Our questions are as follows:

- Are the empty rows for total trips and in/out trips for campground/RV park (highlighted below) intentionally empty, or is there data that should be shown there?
- Is it possible to show the total trips from the proposed improvements as "proposed use totals" rather than "existing use totals" to make it clear that they are trips resulting from the proposed changes?
- It appears that the analysis included both Marina (420) and Public Park (411) codes to analyze trips from the new boat ramp parking spaces is it possible to revise the analysis to include one or the other? As it stands now, both are included in the total trips measurements, which means we are accounting for the new parking lot traffic twice in the calculations. Please see the request from the original email copied below we were hoping to provide two alternative codes to measure the trips from the parking lot based on which ODOT felt was most appropriate, rather than adding the trips from both methods.

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mank you very	y much for you	If fieth: Pleas	e let us know	ii you i	nave any questions.

Best,

Cass

- Marina (420) or Public Park (411) for the 32 new boat ramp parking spaces proposed.
 - o If using Marina (420) 32 "berths" to represent the 32 boat trailer parking spaces
 - If using Public Park (411) 96 "daily trail users", conservatively assuming an average of 3 boat ramp users for each of the 32 new parking spaces

						Rates				To	otal Tri	ps				In/Ou	it Trips	(
				1	Veekda	1	Wee	kend	V	Veekda	у	Wee	kend		Wee	kday		Weel
ITE Code Land Use Descr	Independent iption Variable	No. of Units	Avg Rate or Eq	Daily Rate	AM Peak Rate	PM Peak Rate	Daily Rate	Peak Rate	Daily Trips		PM Peak Trips	Daily Trips		AM Trips	AM Trips Out	PM Trips In	PM Trips Out	Peak Trips In
4:16 Campgrounc/RV	Park Acre(s)	21	Avg		0.48	0.98				10	21			4	6	14	7	10.0
420 Marina	∃erth(s)	32	Avg	2.41	0.07	0.21	2.61	0.22	78	2	7	84	7	1	1	4	3	3
411 Public Park	Acre(s)	96	Avq	0.78	0.02	0.11	1.96	0.28	76	2	11	190	27	1	1	6	5	15
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200																		
		N. C.																
			9					e Totals										

CHANGE OF HEE FUALHATION

From: VYMAZAL Zdenek G <Zdenek.G.VYMAZAL@odot.oregon.gov>

Sent: Thursday, January 11, 2024 11:01 AM

To: Cassandra Dobson < CDobson@parametrix.com>

Subject: RE: Nehalem Bay State Park Improvements ITE Codes

Hello Cassandra,

After reviewing the new data for the proposed development (See attachment snipp), the number of peak trips will not increase to reach fifty (50) trips or more and average daily trip will not increase by five hundred (500) trips or more from the property's prior use as stated in OAR Ch734, Div51 (734-051-3020). It will not requires the Change of Use and such you do not need a traffic study for this proposed development.

However, because the proposed development will use local streets/ roads and is not connected directly to state highway, it is recommended to work with City of Manzanita (County?) on this and include them with you scopping/development work early.

Thank you

Zdenek "Z" Vymazal, PE, PLS
Development Review Coordinator (Area 1)
ODOT – Region 2
455 Airport Rd. SE, Bldg. B
Salem, OR 97301
(971)-345-1318 Cell/Office
zdenek.g.vymazal@odot.oregon.gov
Hours: 5:30 AM to 2:00 PM Monday – Friday

						Rates				T	otal Tri	ps
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ITE Code Land Use Description	Independent Variable	No. of Units	Avg Rate or Eq	Daily Rate	AM Peak Rate	PM Peak Rate	Daily Rate	Peak Rate	Daily Trips	AM Peak Trips	PM Peak Trips	Dai Trij
416 Campground/RV Park	Acre(s)	21	Avg		0.48	0.98				10	21	
420 Marina	Berth(s)	32	Avg	2.41	0.07	0.21	2.61	0.22	78	2	7	84
411 Public Park	Acre(s)	96	Avg	0.78	0.02	0.11	1.96	0.28	76	2	11	19
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CHANGE OF HEE EVALUE

From: VYMAZAL Zdenek G

Sent: Tuesday, January 2, 2024 2:06 PM

To: Cassandra Dobson < CDobson@parametrix.com>

Subject: RE: Nehalem Bay State Park Improvements ITE Codes

Happy New Year to you Casandra too.

Thank you

I and traffic people will look at your info and replay as soon as possible.

Z

Zdenek "Z" Vymazal, PE, PLS
Development Review Coordinator (Area 1)
ODOT – Region 2
455 Airport Rd. SE, Bldg. B
Salem, OR 97301
(971)-345-1318 Cell/Office
zdenek.g.vymazal@odot.oregon.gov
Hours: 5:30 AM to 2:00 PM Monday – Friday

From: Cassandra Dobson < CDobson@parametrix.com >

Sent: Tuesday, January 2, 2024 8:36 AM

To: VYMAZAL Zdenek G <Zdenek.G.VYMAZAL@odot.oregon.gov>

Cc: JOHNSON Tracy * OPRD <Tracy.JOHNSON@oprd.oregon.gov>; Jennifer Hughes <JHughes@parametrix.com>; Ryan Rudnick <RRudnick@parametrix.com>

Subject: Nehalem Bay State Park Improvements ITE Codes

You don't often get email from cdobson@parametrix.com. Learn why this is important

This message was sent from outside the organization. Treat attachments, links and requests with caution. Be conscious of the information you share if you respond.

Good morning Z, and Happy New Year!

Thank you again for taking the time to meet with us to discuss improvements to Nehalem Bay State Park. As discussed at that meeting, our team has looked into the ITE codes that we feel would best fit the proposed improvements. We would recommend use of the following ITE land use codes in peak hour trip generation calculations for the proposed park uses:

- Campground/RV park (416) for the proposed cabins and campsites, and trip generation based on either occupied campsites or acres
 - o 68 occupied campsites (excluding 8 new hiker/biker sites, as there are inherently no vehicle trips associated with these campsites)
 - 6 new staff cabins (already permitted)
 - Up to 10 new cabins at cabin loop
 - Up to 12 new park & walk-in tent sites
 - Up to 40 new cabins/sites in future loop
 - o 21 acres new campground development

New camping/cabin loop +15 acres
New hiker/biker/tent +5 acres
Old hiker biker -1.5 acres
Alternate cabins +2 acres

Staff cabins

+0.5 acres

21 acres new campground development

- Marina (420) or Public Park (411) for the 32 new boat ramp parking spaces proposed.
 - o If using Marina (420) 32 "berths" to represent the 32 boat trailer parking spaces
 - o If using Public Park (411) 96 "daily trail users", conservatively assuming an average of 3 boat ramp users for each of the 32 new parking spaces

Please let us know if you have any questions or would like to discuss further. Thank you again!

Cass

APPENDIX G.

CRASH DATA

TRANSPORTATION DATA SECTION - CRASH ANAYLYSIS AND REPORTING UNIT

CONTINUOUS SYSTEM CRASH LISTING

009: OREGON COAST Highway 009 ALL ROAD TYPES, MP 43.85 to 43.95 01/01/2018 to 12/31/2022, Both Add and Non-Add mileage

> 1 - 4 of 4 Crash records shown.

S D M																			
SER# P R J S	W DATE	COUNTY	RD# FC CONN#	RD CHAR	INT-TYPE					SPCL USE									
INVEST E A U I C	O DAY	CITY	COMPNT FIRST STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR QTY	MOVE			А	S				
RD DPT E L G N H	R TIME	URBAN AREA	MLG TYP SECOND STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E LICNS	PED			
UNLOC? D C S V L	K LAT	LONG	MILEPNT LRS		(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY	Z E	X RES	LOC	ERROR	ACT EVENT	CAUSE
00145 N N N N	05/25/2018	TILLAMOOK	1 02	ALLEY		N	N	CLR	S-1STOP	01 NONE 0	STRGHT								29
NONE	FR		MN 0	UN	(NONE)	STOP SIGN	N	DRY	REAR	PRVTE	E -W							000	00
N	2P		43.85	04			N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	69 F	OR-Y		026	000	29
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										PSNGR CAR		01 DRVR	INJC	17 F			000	000	00
															OR<25				
										02 NONE 0	STOP							010	0.0
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										PSNGR CAR		UZ PSNG	INJC	00 0	IIK.		000	000	00
00188 N N N N N	N 06/24/2019	TILLAMOOK	1 02	ALLEY		N	N	CLR	S-1STOP	01 NONE 0	STRGHT								27,29
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N	45 43 3.07	-123 54 56.95	000900100800	03	Ü		14	DOBIC	INO	I BNOK CAK		OI DRVR	INOD	15 1	OR > 25		001,001	000	00
00273 N N N N	08/09/2021	TILLAMOOK	1 02	INTER	3-LEG	N	N	CLR	BIKE									110	32,27,02
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															OR<25				

COUNTY ROAD CRASH LISTING

TILLAMOOK COUNTY NECARNEY CITY RD, MP -999.99 to 999.99, 01/01/2018 to 12/31/2022

1-4 of 4 Crash records shown.

31.77

	S D M																			
SER#	P RJS	W DATE	MILEPNT	COUNTY ROADS		INT-TYPE					SPCL USE									
INVEST	E A U I C	O DAY	DIST FROM	FIRST STREET	RD CHAR	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR QTY	MOVE			A	S				
RD DPT	E L G N H	R TIME	INTERSECT	SECOND STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E LICNS	PED			
UNLOC?	D C S V L	K LAT	LONG	LRS	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	то	P# TYPE	SVRTY	E	X RES	LOC	ERROR	ACT EVENT	CAUSE
00120	N N N N	04/19/2021	0.03	NECARNEY CITY RD	INTER	CROSS	N	N	CLR	ANIMAL	01 NONE 9	STRGHT							035	12
NONE		MO			UN		UNKNOWN	N	UNK	OTH	N/A	W -E							000	00
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APPENDIX H.

OPERATIONS CALCULATIONS

Intersection						
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			14/5	14/5		
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	P		7	↑	*	
Traffic Vol, veh/h	390	26	58	322	29	65
Future Vol, veh/h	390	26	58	322	29	65
Conflicting Peds, #/hr	0	0	0	0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	424	28	63	350	32	71
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	452	0	914	438
Stage 1	-	-	-	-	438	-
Stage 2	-	-	-	-	476	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1108	_	303	619
Stage 1	-	-	-	-	650	-
Stage 2	-	-	_	_	625	-
Platoon blocked, %	_	_		_	323	
Mov Cap-1 Maneuver	_	_	1108	_	286	619
Mov Cap-1 Maneuver	_		-	_	412	- 019
Stage 1	-	-	-	_	650	
	-	-	-	-		
Stage 2	-	-	-	-	589	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		1.29		13.29	
HCM LOS	U		20		В	
TIOWI LOO					U	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		536	-	-	1108	-
HCM Lane V/C Ratio		0.191	_	-	0.057	-
HCM Control Delay (s/ve	eh)	13.3	-	_	8.4	-
HCM Lane LOS	,	В	_	-	Α	_
HCM 95th %tile Q(veh)		0.7	-	_	0.2	-
/01110 ((1011)		J.1			J.L	

Intersection						
Int Delay, s/veh	0					
		- CDT	MOT	WEE	001	ODD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	Þ		¥	
Traffic Vol, veh/h	0	82	63	0	0	0
Future Vol, veh/h	0	82	63	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	89	68	0	0	0
IVIVIIIL I IOW	U	03	00	U	U	U
Major/Minor	Major1	<u> </u>	Major2	<u> </u>	Minor2	
Conflicting Flow All	68	0	-	0	158	68
Stage 1	_	-	-	-	68	-
Stage 2	_	-	-	-	89	-
Critical Hdwy	4.12	_	_	_	6.42	6.22
Critical Hdwy Stg 1	-	_	_	_	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	_		3.518	
Pot Cap-1 Maneuver	1533	_	-	_	834	995
•	-	_	_	_	954	-
Stage 1		-				
Stage 2	-	-	-	-	934	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1533	-	-	-	834	995
Mov Cap-2 Maneuver	-	-	-	-	834	-
Stage 1	-	-	-	-	954	-
Stage 2	-	-	-	-	934	-
Approach	EB		WB		SB	
HCM Control Delay, s/	v 0		0		0	
HCM LOS					Α	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		1533	-		-	_
HCM Lane V/C Ratio		-	-	-	_	_
HCM Control Delay (s/	\uph\	0	_	_	_	0
HCM Lane LOS	ven)	A		-	-	A
	\					
HCM 95th %tile Q(veh)	0	-	-	-	-

Intersection						
Int Delay, s/veh	2.4					
	EBT	EDD	WDI	WDT	NDI	NDD
Movement		EBR	WBL	WBT	NBL	NBR
Lane Configurations	100	20	7	100	**	00
Traffic Vol, veh/h	439	33	89	466	38	89
Future Vol, veh/h	439	33	89	466	38	89
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	477	36	97	507	41	97
NA - ' /NA'			M. ' C			
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	513	0	1195	495
Stage 1	-	-	-	-	495	-
Stage 2	-	-	-	-	700	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1052	-	206	574
Stage 1	-	-	-	-	613	-
Stage 2	-	-	-	_	493	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	_	1052	_	187	574
Mov Cap-2 Maneuver	_	_	-	_	318	-
Stage 1	_	_	_	_	613	_
Stage 2	_	_	_	_	447	_
Stage 2	-	_	-	-	441	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		1.41		16.05	
HCM LOS					С	
						14/5-
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		463	-		1052	-
HCM Lane V/C Ratio		0.298	-	-	0.092	-
HCM Control Delay (s/ve	eh)	16.1	-	-	8.8	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh)		1.2	-	-	0.3	-
., ., .,						

Interportion						
Intersection Int Delay, s/veh	0					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ન	Þ		Y	
Traffic Vol, veh/h	0	112	117	0	0	0
Future Vol, veh/h	0	112	117	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	122	127	0	0	0
				_		
	Major1		Major2		Minor2	
Conflicting Flow All	127	0	-	0	249	127
Stage 1	-	-	-	-	127	-
Stage 2	-	-	-	-	122	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1459	-	-	-	740	923
Stage 1	_	-	-	_	899	-
Stage 2	_	-	-	-	904	-
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	1459	_	_	_	740	923
Mov Cap-1 Maneuver	-	_	_	_	740	-
Stage 1	_		_	_	899	_
Stage 2	-	_	_	_	904	_
Slaut Z	-	-	-	-	304	-
3.						
J 44 U 5						
Approach	EB		WB		SB	
Approach			WB 0		SB 0	
Approach HCM Control Delay, s/						
Approach					0	
Approach HCM Control Delay, s/v	v 0	EDI	0	MPT	0 A	0DI -4
Approach HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvm	v 0	EBL	0 EBT	WBT	0 A WBR	SBLn1
Approach HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	v 0	1459	0	WBT -	0 A	SBLn1
Approach HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	v 0	1459 -	0 EBT		0 A WBR	-
Approach HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s/v	v 0	1459 - 0	0 EBT	-	0 A WBR	- - 0
Approach HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	v 0	1459 -	0 EBT -	-	0 A WBR	-

	-	•	•	←	4	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		*	†	**	
Traffic Volume (veh/h)	407	31	69	340	32	72
Future Volume (Veh/h)	407	31	69	340	32	72
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	452	34	77	378	36	80
Pedestrians					1	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					0	
Right turn flare (veh)						
Median type	TWLTL			TWLTL		
Median storage veh)	2			2		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			487		1002	470
vC1, stage 1 conf vol					470	
vC2, stage 2 conf vol					532	
vCu, unblocked vol			487		1002	470
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)					5.4	
tF (s)			2.2		3.5	3.3
p0 queue free %			93		92	87
cM capacity (veh/h)			1065		459	593
Direction, Lane #	EB 1	WB 1	WB 2	NB 1		
Volume Total	486	77	378	116		
Volume Left	0	77	0	36		
Volume Right	34	0	0	80		
cSH	1700	1065	1700	544		
Volume to Capacity	0.29	0.07	0.22	0.21		
	0.29	6	0.22	20		
Queue Length 95th (ft)	0.0	8.6	0.0	13.4		
Control Delay (s/veh)	0.0		0.0			
Lane LOS	0.0	A		B		
Approach LOS	0.0	1.5		13.4		
Approach LOS				В		
Intersection Summary						
Average Delay			2.1			
Intersection Capacity Utiliza	ation		46.2%	IC	U Level o	of Service
Analysis Period (min)			15			

Intersection						
	2.2					
		EDD	WDI	WDT	NDI	NDD
	BT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		**	↑	Y	
•	107	31	69	340	32	72
,	107	31	69	340	32	72
Conflicting Peds, #/hr	0	_ 1	_ 1	_ 0	0	0
	ree	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	9	4	4	2	2
	152	34	77	378	36	80
		_				
Major/Minor Majo			Major2		Minor1	
Conflicting Flow All	0	0	488	0	1002	470
Stage 1	-	-	-	-	470	-
Stage 2	-	-	-	-	531	-
Critical Hdwy	-	-	4.14	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.236	-	3.518	3.318
Pot Cap-1 Maneuver	-	_	1065	-	269	593
Stage 1	_	_	-	_	629	_
Stage 2	_	_	_	_	590	_
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	_	1064	_	249	593
Mov Cap-1 Maneuver	-	_	1004	_	380	-
•	-	-	_	-	628	
Stage 1	-	-	_	_		-
Stage 2	-	-	-	-	547	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		1.46		14.22	
HCM LOS					В	
110 200						
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		505	-		1064	-
HCM Lane V/C Ratio		0.229	-	-	0.072	-
HCM Control Delay (s/veh		14.2	-	-	8.6	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.9	-	-	0.2	-
Tom odd /vallo w(vall)		0.0			0.2	

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ર્ન	1		Y	
Traffic Volume (veh/h)	0	92	78	0	0	0
Future Volume (Veh/h)	0	92	78	0	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	0	105	89	0	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		110110	110110			
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	89				194	89
vC1, stage 1 conf vol	0.5				134	00
vC2, stage 2 conf vol						
vCu, unblocked vol	89				194	89
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)	4.1				0.4	0.2
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	100
					795	969
cM capacity (veh/h)	1506				790	909
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	105	89	0			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1506	1700	1700			
Volume to Capacity	0.00	0.05	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s/veh)	0.0	0.0	0.0			
Lane LOS			Α			
Approach Delay (s/veh)	0.0	0.0	0.0			
Approach LOS			Α			
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		8.6%	IC	U Level c	f Sarvice
	Lation			IU	O Level C	ii Sei Vice
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	P		M	
Traffic Vol, veh/h	0	92	78	0	0	0
Future Vol, veh/h	0	92	78	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	2	5	8	2	2	2
Mvmt Flow	0	105	89	0	0	0
				_		
		_		_		
	Major1		Major2		Minor2	
Conflicting Flow All	89	0	-	0	193	89
Stage 1	-	-	-	-	89	-
Stage 2	-	-	-	-	105	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1507	-	-	-	796	969
Stage 1	-	_	_	-	935	-
Stage 2	_	-	-	-	920	-
Platoon blocked, %		_	-	_		
Mov Cap-1 Maneuver	1507	_	_	_	796	969
Mov Cap-2 Maneuver	-	_	_	_	796	-
Stage 1	_	_	_	_	935	_
Stage 2	_	_	_	_	920	_
Olage 2					320	
Approach	EB		WB		SB	
HCM Control Delay, s/	v 0		0		0	
HCM LOS					Α	
Minor Long/Major My	_4	EDI	EDT	WDT	WDD	ODL =4
Minor Lane/Major Mvn	π	EBL	EBT	WBT	WBR :	SBLILL
Capacity (veh/h)		1507	-	-	-	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s/	veh)	0	-	-	-	0
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q(veh)	0	-	-	-	-

Intersection						
Int Delay, s/veh	2.7					
		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	07	7	107	**	00
Traffic Vol, veh/h	457	37	99	487	42	99
Future Vol, veh/h	457	37	99	487	42	99
Conflicting Peds, #/hr	_ 0	_ 2	_ 2	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	3	2	5	2	3	2
Mvmt Flow	491	40	106	524	45	106
NA -1 - /NA1			M. ' C		M	
	lajor1		Major2		Minor1	
Conflicting Flow All	0	0	533	0	1250	513
Stage 1	-	-	-	-	513	-
Stage 2	-	-	-	-	737	-
Critical Hdwy	-	-	4.15	-	6.43	6.22
Critical Hdwy Stg 1	-	-	-	-	5.43	-
Critical Hdwy Stg 2	-	-	-	-	5.43	-
Follow-up Hdwy	-	-	2.245	-	3.527	3.318
Pot Cap-1 Maneuver	-	-	1019	-	190	561
Stage 1	_	_	-	_	599	-
Stage 2	_	_	_	-	472	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	_	1017	_	170	560
Mov Cap-1 Maneuver	_		1011	_	300	-
Stage 1	-	-	_	_	598	-
	-	-	-	-	422	
Stage 2	-	-	-	-	422	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		1.51		17.21	
HCM LOS					C	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		445	-	-	1017	-
HCM Lane V/C Ratio		0.341	-	-	0.105	-
HCM Control Delay (s/v	eh)	17.2	-	-	9	-
HCM Lane LOS	,	С	-	-	Α	-
HCM 95th %tile Q(veh)		1.5	-	-	0.3	-
2000						

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ર્સ	1		M	
Traffic Volume (veh/h)	0	125	130	0	0	0
Future Volume (Veh/h)	0	125	130	0	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	136	141	0	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	141				277	141
vC1, stage 1 conf vol	, , ,					
vC2, stage 2 conf vol						
vCu, unblocked vol	141				277	141
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)	7.1				J. .	٧.٢
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	100
cM capacity (veh/h)	1442				713	907
					7 10	301
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	136	141	0			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1442	1700	1700			
Volume to Capacity	0.00	0.08	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s/veh)	0.0	0.0	0.0			
Lane LOS			Α			
Approach Delay (s/veh)	0.0	0.0	0.0			
Approach LOS			Α			
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliza	ation		10.8%	IC	ULevelo	f Service
Analysis Period (min)			15.07	.0	2 23101 0	
Analysis i chou (min)			10			

Interpostion						
Intersection Int Delay, s/veh	0					
·						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	Þ		Y	
Traffic Vol, veh/h	0	125	130	0	0	0
Future Vol, veh/h	0	125	130	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	3	7	2	2	2
Mvmt Flow	0	136	141	0	0	0
			• • •	•	•	
	//ajor1		Major2		Minor2	
Conflicting Flow All	141	0	-	0	277	141
Stage 1	-	-	-	-	141	-
Stage 2	-	-	-	-	136	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1442	-	-	-	713	907
Stage 1	_	-	-	-	886	-
Stage 2	-	-	-	-	891	-
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	1442	_	_	-	713	907
Mov Cap-2 Maneuver	-	_	_	_	713	-
Stage 1	_	-	_	_	886	_
Stage 2	-	_	_	_	891	_
Olaye Z	_	-	-	-	091	
			WB		SB	
Approach	EB		,,,,			
			0		0	
Approach HCM Control Delay, s/v HCM LOS					0 A	
HCM Control Delay, s/v						
HCM Control Delay, s/v HCM LOS	<i>,</i> 0	EDI	0	WDT	Α	ODI 4
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvmt	<i>,</i> 0	EBL	0 EBT	WBT	A WBR	SBLn1
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	<i>,</i> 0	1442	0	WBT -	Α	SBLn1 -
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	t 0	1442	0 EBT		A WBR	-
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s/v	t 0	1442 - 0	0 EBT	-	A WBR S	- - 0
HCM Control Delay, s/v HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	t veh)	1442	0 EBT -	- -	A WBR S	-

	-	•	•	•	4	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f >		*	†	**	
Traffic Volume (veh/h)	407	34	75	340	35	77
Future Volume (Veh/h)	407	34	75	340	35	77
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	438	37	81	366	38	83
Pedestrians	100	O.	Ŭ.	000	2	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					0.0	
Right turn flare (veh)					U	
	TWLTL			TWLTL		
Median type	2			2		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked			477		007	450
vC, conflicting volume			477		987	459
vC1, stage 1 conf vol					459	
vC2, stage 2 conf vol			477		528	450
vCu, unblocked vol			477		987	459
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)					5.4	
tF (s)			2.2		3.5	3.3
p0 queue free %			92		92	86
cM capacity (veh/h)			1068		460	601
Direction, Lane#	EB 1	WB 1	WB 2	NB 1		
Volume Total	475	81	366	121		
Volume Left	0	81	0	38		
Volume Right	37	0	0	83		
cSH	1700	1068	1700	549		
Volume to Capacity	0.28	0.08	0.22	0.22		
Queue Length 95th (ft)	0	6	0	21		
Control Delay (s/veh)	0.0	8.6	0.0	13.4		
Lane LOS		Α		В		
Approach Delay (s/veh)	0.0	1.6		13.4		
Approach LOS				В		
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utiliza	ation		47.3%	IC	III evel d	of Service
Analysis Period (min)	46011		15	10	O LOVOI C	, COI VIOG
Analysis Penou (min)			10			

Intersection						
Int Delay, s/veh	2.3					
	ВТ	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ		*	↑	Y	
•	407	34	75	340	35	77
·	407	34	75	340	35	77
Conflicting Peds, #/hr	0	2	2	0	0	0
Sign Control F	ree	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	3	2	5	2	3	2
	438	37	81	366	38	83
NA : (NA: NA:					\d.	
Major/Minor Maj			Major2		Minor1	
Conflicting Flow All	0	0	476	0	985	458
Stage 1	-	-	-	-	458	-
Stage 2	-	-	-	-	527	-
Critical Hdwy	-	-	4.15	-	6.43	6.22
Critical Hdwy Stg 1	-	-	-	-	5.43	-
Critical Hdwy Stg 2	-	-	-	-	5.43	-
Follow-up Hdwy	-	-	2.245	-	3.527	3.318
Pot Cap-1 Maneuver	-	-	1070	-	274	603
Stage 1	-	-	-	-	635	-
Stage 2	-	-	-	-	590	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1068	-	253	602
Mov Cap-2 Maneuver	_	_	-	_	382	_
Stage 1	-	_	_	_	634	_
Stage 2	_	-	_	_	546	_
otago 2					0.0	
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		1.56		14.23	
HCM LOS					В	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
	- 1					
Capacity (veh/h)		510	-		1068	-
HCM Control Polocy (a/col	- \	0.236	-		0.075	-
HCM Control Delay (s/veh	1)	14.2	-	-	8.6	-
HCM Lane LOS		В	-	-	A	-
HCM 95th %tile Q(veh)		0.9	-	-	0.2	-

	•	→	—	4	-	1	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ર્સ	1		M		
Traffic Volume (veh/h)	6	92	78	9	8	5	
Future Volume (Veh/h)	6	92	78	9	8	5	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	7	100	85	10	9	5	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	95				204	90	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	95				204	90	
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	100				99	99	
cM capacity (veh/h)	1499				781	968	
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	107	95	14				
Volume Left	7	0	9				
Volume Right	0	10	5				
cSH	1499	1700	839				
Volume to Capacity	0.00	0.06	0.02				
Queue Length 95th (ft)	0.00	0.00	1				
• , ,	0.5	0.0	9.4				
Control Delay (s/veh)	0.5 A	0.0	9.4 A				
Lane LOS Approach Delay (s/veh)	0.5	0.0	9.4				
Approach LOS	0.5	0.0	9.4 A				
			А				
Intersection Summary							
Average Delay			0.9				
Intersection Capacity Utilizati	on		20.6%	IC	U Level o	of Service	
Analysis Period (min)			15				

Intersection						
Int Delay, s/veh	0.8					
		EST	MOT	\A/B-B	051	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	Þ		A	
Traffic Vol, veh/h	6	92	78	9	8	5
Future Vol, veh/h	6	92	78	9	8	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	е,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	3	7	2	2	2
Mvmt Flow	7	100	85	10	9	5
			_			
	Major1		//ajor2		Minor2	
Conflicting Flow All	95	0	-	0	203	90
Stage 1	-	-	-	-	90	-
Stage 2	-	-	-	-	113	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1499	-	-	-	786	968
Stage 1	-	_	-	-	934	-
Stage 2	-	-	-	-	912	-
Platoon blocked, %		_	_	_	V 12	
Mov Cap-1 Maneuver	1499	_	_	_	782	968
Mov Cap-1 Maneuver	-		_	_	782	-
Stage 1	-	_	-	-	930	-
•	_	-	-	-	912	-
Stage 2	-	-	-	-	312	-
Approach	EB		WB		SB	
HCM Control Delay, s/	v 0.45		0		9.33	
HCM LOS					Α	
J = 0 0						
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR :	
Capacity (veh/h)		110	-	-	-	
HCM Lane V/C Ratio		0.004	-	-		0.017
HCM Control Delay (s/	/veh)	7.4	0	-	-	9.3
HCM Lane LOS		Α	Α	-	-	Α
HCM 95th %tile Q(veh)	0	-	-	-	0.1

	→	•	•	•	1	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f >		*	↑	¥	
Traffic Volume (veh/h)	457	40	104	487	44	102
Future Volume (Veh/h)	457	40	104	487	44	102
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	491	43	112	524	47	110
Pedestrians					2	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					0	
Right turn flare (veh)						
Median type	TWLTL			TWLTL		
Median storage veh)	2			2		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			536		1263	515
vC1, stage 1 conf vol					515	
vC2, stage 2 conf vol					748	
vCu, unblocked vol			536		1263	515
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)					5.4	
tF (s)			2.2		3.5	3.3
p0 queue free %			89		87	80
cM capacity (veh/h)			1015		362	559
Direction, Lane #	EB 1	WB 1	WB 2	NB 1		
Volume Total	534	112	524	157		
Volume Left	0	112	0	47		
Volume Right	43	0	0	110		
cSH	1700	1015	1700	481		
Volume to Capacity	0.31	0.11	0.31	0.33		
Queue Length 95th (ft)	0.01	9	0.01	35		
Control Delay (s/veh)	0.0	9.0	0.0	16.1		
Lane LOS	0.0	Α.	0.0	C		
Approach Delay (s/veh)	0.0	1.6		16.1		
Approach LOS	0.0	1.0		C		
• •				0		
Intersection Summary			^-			
Average Delay			2.7			
Intersection Capacity Utiliz	ation		54.5%	IC	U Level o	f Service
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	2.8					
<u> </u>		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	4.0	*	↑	Y	400
Traffic Vol, veh/h	457	40	104	487	44	102
Future Vol, veh/h	457	40	104	487	44	102
Conflicting Peds, #/hr	0	2	2	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	3	2	5	2	3	2
Mvmt Flow	491	43	112	524	47	110
		_				
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	536	0	1262	515
Stage 1	-	-	-	-	515	-
Stage 2	-	-	-	-	747	-
Critical Hdwy	-	-	4.15	-	6.43	6.22
Critical Hdwy Stg 1	-	-	-	-	5.43	-
Critical Hdwy Stg 2	-	-	-	-	5.43	-
Follow-up Hdwy	-	-	2.245	-	3.527	3.318
Pot Cap-1 Maneuver	_	-	1017	-	187	560
Stage 1	_	_	-	-	598	_
Stage 2	_	-	_	_	466	_
Platoon blocked, %	_	_		_	.00	
Mov Cap-1 Maneuver	_	_	1015	_	166	559
Mov Cap-1 Maneuver	_		-		295	-
	-	-	-	-	597	
Stage 1	-	-	_	-		
Stage 2	-	-	-	-	415	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		1.58		17.63	
HCM LOS	-				С	
110111 200					, i	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		440	-	-	1015	-
HCM Lane V/C Ratio		0.356	-	-	0.11	-
HCM Control Delay (s/ve	eh)	17.6	-	-	9	-
HCM Lane LOS	,	С	-	-	Α	-
HCM 95th %tile Q(veh)		1.6	-	-	0.4	-
1.5.77 55th 75th Q(VOII)		1.0			0.7	

	٠	→	+	•	-	1
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	1		M	
Traffic Volume (veh/h)	8	125	130	8	5	5
Future Volume (Veh/h)	8	125	130	8	5	5
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	136	141	9	5	5
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	150				300	146
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	150				300	146
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF(s)	2.2				3.5	3.3
p0 queue free %	99				99	99
cM capacity (veh/h)	1431				688	902
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	145	150	10			
Volume Left	9	0	5			
Volume Right	0	9	5			
cSH	1431	1700	780			
Volume to Capacity	0.01	0.09	0.01			
	0.01	0.09	1			
Queue Length 95th (ft) Control Delay (s/veh)	0.5	0.0	9.7			
		0.0				
Lane LOS	Α	0.0	A			
Approach LOS	0.5	0.0	9.7			
Approach LOS			Α			
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilizat	ion		24.3%	IC	U Level c	of Service
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.6					
		FRT	MOT	ME	051	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्भ	ħ		A	_
Traffic Vol, veh/h	8	125	130	8	5	5
Future Vol, veh/h	8	125	130	8	5	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	3	7	2	2	2
Mvmt Flow	9	136	141	9	5	5
Mailan/Minan	Ma:4		4-:0		A:O	
	Major1		Major2		Minor2	4.40
Conflicting Flow All	150	0	-	0	299	146
Stage 1	-	-	-	-	146	-
Stage 2	-	-	-	-	153	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1431	-	-	-	692	901
Stage 1	-	-	-	-	882	-
Stage 2	-	-	-	-	875	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1431	-	-	-	688	901
Mov Cap-2 Maneuver		-	-	-	688	-
Stage 1	-	-	-	-	876	-
Stage 2	-	_	_	_	875	_
					- · •	
Approach	EB		WB		SB	
HCM Control Delay, sa	/v 0.45		0		9.68	
HCM LOS					Α	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR :	SRI n1
Capacity (veh/h)	111	108			- 1001	780
HCM Lane V/C Ratio			-	-		0.014
	/vob)	0.006	-	-		
HCM Control Delay (s. HCM Lane LOS	(Veii)	7.5	0	-	-	9.7
	.\	A	Α	-	-	A
HCM 95th %tile Q(veh	1)	0	-	-	-	0

APPENDIX I. **QUEUING ANALYSIS**

Movement	WB	NB
Directions Served	L	LR
Maximum Queue (ft)	79	89
Average Queue (ft)	22	38
95th Queue (ft)	55	70
Link Distance (ft)		1129
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	100	
Storage Blk Time (%)	0	
Queuing Penalty (veh)	0	

Intersection: 2: Necarney City Rd & Loop Rd

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 0

1 EXPM Manzanita Pines SimTraffic Report CNL Page 1

Movement	WB	NB
Directions Served	L	LR
Maximum Queue (ft)	85	122
Average Queue (ft)	32	52
95th Queue (ft)	65	98
Link Distance (ft)		1129
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	100	
Storage Blk Time (%)	0	
Queuing Penalty (veh)	0	

Intersection: 2: Necarney City Rd & Loop Rd

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 0

2 EXSAT Manzanita Pines SimTraffic Report CNL Page 1

Movement	EB	WB	NB
Directions Served	TR	L	LR
Maximum Queue (ft)	4	66	99
Average Queue (ft)	0	26	37
95th Queue (ft)	3	59	68
Link Distance (ft)	1178		1129
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)		100	
Storage Blk Time (%)		0	
Queuing Penalty (veh)		0	

Intersection: 2: Necarney City Rd & Loop Rd

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 0

3 PREPM Manzanita Pines

CNL

SimTraffic Report
Page 1

Movement	WB	NB
Directions Served	L	LR
Maximum Queue (ft)	89	143
Average Queue (ft)	35	60
95th Queue (ft)	71	116
Link Distance (ft)		1129
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	100	
Storage Blk Time (%)	0	
Queuing Penalty (veh)	0	

Intersection: 2: Necarney City Rd & Loop Rd

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Network Summary

Network wide Queuing Penalty: 0

4 PRESAT Manzanita Pines SimTraffic Report CNL Page 1

Movement	WB	NB
Directions Served	L	LR
Maximum Queue (ft)	71	131
Average Queue (ft)	26	44
95th Queue (ft)	58	89
Link Distance (ft)		1129
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	100	
Storage Blk Time (%)	0	
Queuing Penalty (veh)	0	

Intersection: 2: Necarney City Rd & Loop Rd

Movement	SB
Directions Served	LR
Maximum Queue (ft)	36
Average Queue (ft)	9
95th Queue (ft)	34
Link Distance (ft)	370
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 0

5 POSTPM Manzanita Pines SimTraffic Report CNL Page 1

Movement	EB	WB	NB
Directions Served	TR	L	LR
Maximum Queue (ft)	5	94	147
Average Queue (ft)	0	35	56
95th Queue (ft)	4	73	110
Link Distance (ft)	1178		1129
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)		100	
Storage Blk Time (%)		0	
Queuing Penalty (veh)		1	

Intersection: 2: Necarney City Rd & Loop Rd

Movement	EB	SB
Directions Served	LT	LR
Maximum Queue (ft)	14	35
Average Queue (ft)	1	9
95th Queue (ft)	9	35
Link Distance (ft)	1054	370
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 1

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